GENERAL STRUCTURAL NOTES

GENERAL

- CONTRACTOR IS TO FIELD VERIFY ALL DIMENSIONS, AND ELEVATIONS AS SHOWN ON CONTRACT DRAWINGS.
 ANY DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF WR&A. FAILURE TO COMPLY MAY RESULT IN
 REWORK AT CONTRACTOR'S EXPENSE.
- 2. THE SIZES AND LOCATIONS OF EQUIPMENT SUPPORTS AND EQUIPMENT-RELATED FLOOR AND WALL OPENINGS, ARE DEPENDENT ON THE ACTUAL EQUIPMENT FURNISHED. CONTRACTOR SHALL VERIFY AND COORDINATE ALL SUCH ITEMS BEFORE PROCEEDING WITH WORK. DIMENSIONS INDICATED ON THESE DRAWINGS SHALL NOT BE ALTERED WITHOUT APPROVAL FROM WR&A. STRUCTURAL DRAWINGS MAY NOT SHOW ALL EQUIPMENT PADS AND OTHER EQUIPMENT SUPPORTS REQUIRED. REFER TO AND WORK WITH CIVIL, ARCHITECTURAL, MECHANICAL AND ELECTRICAL DRAWINGS AND MANUFACTURER'S SPECIFICATIONS, DIAGRAMS AND INSTALLATION INSTRUCTIONS BEFORE PROCEEDING WITH WORK. QUESTIONS CAN BE DIRECTED TO WR&A.
- 3. THE LOCATION OF BORINGS CONDUCTED BY "DRAPER ADEN ASSOCIATES" ARE SHOWN IN THE SOILS REPORT. BORING LOGS ARE INCLUDED IN THE SPECIFICATIONS.
- 4. CONTRACTOR SHALL BE RESPONSIBLE FOR COORDINATING ALL CONSTRUCTION ACTIVITIES, INCLUDING THOSE OF ALL SUBCONTRACTORS.

FOUNDATION NOTES:

- 1. KEEP ALL EXCAVATIONS DRY. STANDING WATER IN EXCAVATED AREAS IS NOT ALLOWED. PROVIDE A LAYER OF 6 MIL VAPOR BARRIER AND A 6" LAYER OF VDOT #57 STONE UNDER ALL SLABS ON GRADE AND 6" LAYER OF COMPACTED CRUSHED STONE UNDER SPREAD FOOTINGS.
- 2. COORDINATE MECHANICAL AND ELECTRICAL WORK PRIOR TO PROCEEDING WITH FOUNDATION WORK, SEE MECHANICAL AND ELECTRICAL DRAWINGS.
- 3. SOIL BEARING CAPACITY FOR WHICH ALL SPREAD FOOTINGS HAVE BEEN DESIGNED IS A MAXIMUM OF 3500 PSF PER RECOMMENDATIONS OF THE GEOTECHNICAL ENGINEER.

CONCRETE NOTES:

- 1. PROVIDE CONCRETE WITH A MINIMUM COMPRESSIVE STRENGTH OF 4000 PSI AT 28 DAYS.
- 2. DETAIL AND CONSTRUCT REINFORCED CONCRETE IN ACCORDANCE WITH AMERICAN CONCRETE INSTITUTE ACI 301, "SPECIFICATION FOR STRUCTURAL CONCRETE."
- 3. DETAIL REINFORCING STEEL IN ACCORDANCE WITH AMERICAN CONCRETE INSTITUTE ACI 315, "MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCED CONCRETE STRUCTURES," AND ACI SP-66, "ACI DETAILING MANUAL."
- 4. PROVIDE REINFORCEMENT CONFORMING TO ASTM A 615, GRADE 60, DEFORMED BARS.
- PROVIDE WELDED WIRE FABRIC CONFORMING TO ASTM A 185.
- 6. UNLESS NOTED OTHERWISE ON THE DRAWINGS, THE CONCRETE COVER FOR REINFORCEMENT SHALL BE AS FOLLOWS:
 - A. BOTTOM BARS IN FOOTINGS AND IN SLABS ON EARTH OR CRUSHED STONE: 3".
 - B. BEAMS, SLABS, COLUMNS AND WALLS EXPOSED TO GROUND, WEATHER AFTER REMOVAL OF FORMS: 2"
 - C. BEAMS, COLUMNS, WALLS AND PIERS NOT EXPOSED TO WEATHER: 1-1/2".
 - D. STRUCTURAL SLABS NOT EXPOSED TO GROUND, WEATHER OR TRUCK TRAFFIC: 1".
 - E. STRUCTURAL SLABS NOT EXPOSED TO GROUND, WEATHER, PROCESS LIQUID OR VAPORS, BUT SUBJECT TO TRUCK TRAFFIC:

 TOP OF SLAB: 1-1/2".

TOP OF SLAB: 1-1/2". BOTTOM OF SLAB: 1".

- 7. SUBMIT REINFORCING STEEL DETAILS (SHOP DRAWINGS) AND RECEIVE APPROVAL BEFORE PROCEEDING WITH FABRICATION.
- 8. CHAMFER ALL EXPOSED CONCRETE EDGES 3/4" UNLESS OTHERWISE NOTED.
- DETAIL ALL SPLICES FOR REINFORCING BARS NOT DIMENSIONED ON DRAWINGS AS TABULATED ON THIS DRAWING.
- 10. POUR CONCRETE SLABS AND WALLS BETWEEN INDICATED JOINTS, ALLOWING A MINIMUM ELAPSED PERIOD OF 3 DAYS BETWEEN ADJACENT POURS.
- 11. PROVIDE JOINTS AS DETAILED ON THE DRAWINGS. NO ADDITIONAL JOINTS SHALL BE USED NOR ANY OMITTED EXCEPT BY WRITTEN AUTHORIZATION FROM WR&A. APPROVED ADDITIONAL JOINTS SHALL NOT RESULT IN ADDITIONAL EXPENSE TO THE OWNER.
- 12. PROVIDE 3/8" THICK PVC, 9" WIDE WATERSTOPS OF THE TWO-BULB TYPE AS SHOWN ON THE DRAWINGS.
- 13. SIZE AND LOCATE ANCHOR BOLTS AND EQUIPMENT PADS OR PEDESTALS TO SUIT EQUIPMENT FURNISHED.
- 14. REVIEW ALL DRAWINGS FROM OTHER DISCIPLINES AND COORDINATE ALL OPENINGS AND EMBEDDED ITEMS SUCH AS SLEEVES, ANCHORS, CONDUIT, ETC. THAT WILL BE INCORPORATED INTO CONCRETE WORK.
- 15. WHERE A CONCRETE GRADE BEAM FRAMES INTO A WALL OR PIER, IF A CONSTRUCTION JOINT IS NOT INDICATED AT THE BOTTOM OF THE BEAM, PROVIDE A POCKET IN THE WALL OR PIER FOR BEAM BEARING. DEPTH OF POCKET SHALL BE A MINIMUM OF 4 INCHES.

CONCRETE MASONRY NOTES:

- CONSTRUCT MASONRY IN ACCORDANCE WITH AMERICAN CONCRETE INSTITUTE ACI-530/ ASCE 5/ TMS 402, LATEST EDITION "BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES".
- PROVIDE MORTAR CONFORMING TO THE REQUIREMENTS OF ASTM C-270, TYPE M OR S. CEMENT USED FOR MORTAR SHALL BE PORTLAND CEMENT.
- PROVIDE GROUT CONFORMING TO THE REQUIREMENTS OF ASTM C 476 COARSE GROUT, WITH A MINIMUM COMPRESSIVE STRENGTH OF 2.000 PSI AT 28 DAYS.
- 4. PROVIDE CONCRETE MASONRY UNITS WITH A MINIMUM COMPRESSIVE STRENGTH (F'm) OF 1,500 PSI.
- 5. PROVIDE REINFORCING BARS CONFORMING TO ASTM A 615, GRADE 60.
- 6. MASONRY SHALL COMPLY WITH THE TYPICAL DETAILS FOR CONCRETE MASONRY WALL REINFORCING AND CONCRETE MASONRY OPENING REINFORCING AS SHOWN ON S0.3.
- ALL EXT. MASONRY WALLS SHALL BE REINF. WITH #5 AT 24" CENTERED ON FULLY GROUTED CELL, TYP. U.O.N.

STRUCTURAL STEEL NOTES:

- 1. FABRICATE AND ERECT STRUCTURAL STEEL CONFORMING TO THE REQUIREMENTS OF AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC), "MANUAL OF STEEL CONSTRUCTION".
- 2. SUBMIT ERECTION PLANS AND SHOP DETAILS AND RECEIVE APPROVAL BEFORE PROCEEDING WITH FABRICATION. FAILURE TO COMPLY MAY RESULT IN CORRECTIVE WORK AT CONTRACTOR'S EXPENSE.
- 3. PROVIDE STRUCTURAL STEEL WIDE FLANGE SHAPES CONFORMING TO ASTM A992 (Fy=50KSI), HSS MEMBERS CONFORMING TO ASTM A500 (Fy=46KSI), AND STEEL PLATES AND OTHERS CONFORMING TO ASTM A36 (Fy=36KSI)
- 4. PROVIDE ANCHOR BOLTS CONFORMING TO ASTM F1554 (FyMIN=36KSI).
- 5. PROVIDE HIGH STRENGTH BOLTS CONFORMING TO ASTM A325. PROVIDE ALL BOLTED CONNECTIONS WITH COMPRESSIBLE-WASHER-TYPE DIRECT-TENSION INDICATORS CONFORMING TO ASTM F959.
- 6. WELD IN COMPLIANCE WITH AMERICAN WELDING SOCIETY AWS D1.1, "STRUCTURAL WELDING CODE"
- 7. WELD ALL SHOP CONNECTIONS WITH CLASS E-70 SERIES ELECTRODES. PROVIDE FIELD CONNECTIONS WITH HIGH STRENGTH BOLTED CONNECTIONS EXCEPT WHERE NOTED.
- 8. ALL FULL PENETRATION WELDS SHALL FOLLOW THE MEANS AND METHODS OUTLINED IN THE AWS D1.1 WELDING CODE.
- 9. CONNECTIONS NOT INDICATED SHALL BE DESIGNED BY THE FABRICATOR. DESIGN BEAM CONNECTIONS FOR ONE HALF OF THE TOTAL ALLOWABLE UNIFORM LOAD FOR THE SPAN OF THE BEAM AS GIVEN IN PART 2 OF THE "MANUAL OF STEEL CONSTRUCTION". USE THE VALUES IN TABLE II OF AISC MANUAL "ALLOWABLE STRESS DESIGN OF SIMPLE SHEAR CONNECTIONS". USE THE GREATEST NUMBER OF BOLTS ALLOWED FOR THE DEPTH OF THE BEAMS USING A MINIMUM YIELD OF 50 KSI.
- 10. PROVIDE MINIMUM CONNECTION OF TWO 3/4" DIAMETER HIGH STRENGTH STEEL BOLTS OR WELDS OF EQUIVALENT STRENGTH COMPLYING WITH THE CONNECTION REQUIREMENTS DELINEATED HEREIN.
- 11. MILL BOTTOM OF ALL COLUMNS AND FINISH TOP OF ALL BASE PLATES IN ACCORDANCE WITH AISC SPECIFICATIONS. WELD BASE PLATES TO BOTTOM OF COLUMNS.
- 12. CONTRACTOR IS REQUIRED TO SHORE ALL GIRDERS BEFORE PLACING CONCRETE ON ELEVATED SLABS, TYP.

CODES AND STANDARDS:

- INTERNATIONAL BUILDING CODE (IBC) 2003 INCLUDING THE MODIFICATIONS MADE BY LOCAL JURISDICTION.
- 2. AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC) "MANUAL OF STEEL CONSTRUCTION NINTH EDITION-ALLOWABLE STRESS DESIGN."
- 3. AMERICAN CONCRETE INSTITUTE ACI-318 2005, "BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE."
- 4. AMERICAN CONCRETE INSTITUTE ACI-530/ ASCE 5/ TMS 402, 2005 EDITION "BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES".
- 5. AMERICAN SOCIETY OF CIVIL ENGINEERS ASCE 7 2005, "MINIMUM DESIGN LOADS FOR BUILDING AND OTHER STRUCTURES."
- 6. AMERICAN ASSOCIATION OF STATE HIGHWAY AND TRANSPORTATION OFFICIALS (AASHTO) "STANDARD

SPECIFICATIONS FOR HIGHWAY BRIDGES," LATEST EDITION.

FEDERAL EMERGENCY MANAGEMENT AGENCY (FEMA), FEMA-353 2000, "RECOMMENDED SPECIFICATIONS AND QUALITY ASSURANCE GUIDELINES FOR STEEL MOMENT-FRAME CONSTRUCTION FOR SEISMIC APPLICATIONS".

	LAP SPLICE LENGTH				N	MINIMUM TENSI	ON EMBEDMEN	Т
BAR SIZE	SLAB AN	ID WALL	BE	AM	STD 90 H	STD 90 HOOK		HOOK
ENGLISH	OTHER BARS	TOP BARS	OTHER BARS	TOP BARS	ldh	12db	ldh	4db
#3	12"	16"	13"	18"	6"	5"	6"	2 1/2"
#4	15"	20"	22"	28"	7"	6"	7"	2 1/2"
#5	20"	24"	32"	41"	9"	8"	9"	3"
#6	22"	30"	40"	52"	10"	9"	10"	3"
#7	37"	48"	59"	76"	12"	11"	12"	4"
#8	48"	60"	68"	89"	14"	12"	14"	4"
#9	58"	74"	77"	101"	15"	14"	15"	5"
#10	70"	91"	88"	114"	17"	16"	17"	5"
#11	83"	108"	98"	128"	19"	17"	19"	6"

STEEL DECK NOTES:

- 1. FABRICATE AND ERECT ALL STEEL DECK CONFORMING TO THE REQUIREMENTS OF "STEEL DECK INSTITUTE" (SDI), DESIGN MANUAL FOR COMPOSITE DECKS, FORM DECKS AND ROOF DECKS NO.31.
- 2. STEEL DECK SHALL BE FABRICATED OF G-90 GALVANIZED SHEETS CONFORMING TO ASTM C53-06a, STRUCTURAL QUALITY. AND FEDERAL SPECIFICATION QQ-S-775.
- STEEL DECK SHALL BE 2VLI18 BY "VULCRAFT" OR APPROVED EQUAL FOR 1ST AND 2ND FLOOR AREAS. STEEL DECK SHALL BE 1.5VL18 BY "VULCRAFT" OR APPROVED EQUAL FOR ATTIC AREAS. STEEL DECK SHALL BE 1.5B18 BY "VULCRAFT" OR APPROVED EQUAL FOR ROOF AREAS.
- 4. SUBMIT ERECTION PLANS AND SHOP DETAILS AND RECEIVE APPROVAL BEFORE PROCEEDING WITH FABRICATION AND ERECTION. FAILURE TO COMPLY MAY RESULT IN CORRECTIVE WORK AT CONTRACTOR'S EXPENSE.
- 5. STAGGERING OF STEEL DECK END LAPS IS NOT PERMITTED.
- 6. DECK ENDS MAY BE EITHER BUTTED OR LAPPED OVER SUPPORTS
- b) ALL OTHER ROOF AREAS 30 PSF
- 8. THE LOCATION AND NUMBER OF FASTENERS REQUIRED TO SATISFY ATTACHMENT OF DECK TO STEEL MEMBERS SHALL BE AS FOLLOWS:
 - a) 36/4 PATTERN AT ALL SUPPORTS.
 - b) SIDE LAPS AND PERIMETER EDGES SHALL BE ATTACHED AT 36 INCHES MAXIMUM SPACING, BUT WITH A MINIMUM OF 1 FASTENER AT MID-SPAN.
- 9. ALL FIELD WELDING OF DECK SHALL BE IN STRICT ACCORDANCE WITH AWS D1.3 STRUCTURAL WELDING CODE SHEET METAL.
- A MINIMUM VISIBLE 5/8 INCH DIAMETER PUDDLE WELDS OR AN ELONGATED WELD WITH AN EQUAL PERIMETER IS REQUIRED. FILLET WELDS, WHEN USED, SHALL BE AT LEAST 1 INCH LONG.

DESIGN LOADS:

1. DEAD LOADS:

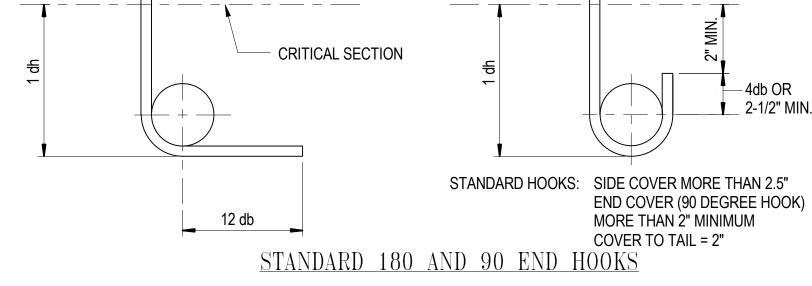
- A. STRUCTURES: ACTUAL WEIGHT
- B. WEIGHT OF SOIL: 100 PCF FOR RESISTING UPLIFT AND BUOYANCY
 C. WEIGHT OF SOIL: 120 PCF FOR DEAD LOAD
- D. MECHANICAL/ELECTRICAL/PLUMBING: 30 PSF.
- 2. LIVE LOADS:
 - A. FLOORS: 100 PSF IN OFFICE AREAS & 200 PSF IN AREAS OCCUPIED BY EQUIPMENT.
 - B. EQUIPMENT: ACTUAL WEIGHT OF EQUIPMENT.
 - C. WALKWAYS AND STAIRWAYS: 100 PSF.
 - D. ROOF: 20 PSF.
 E. SNOW LOAD: GROUND SNOW LOAD (Pg): 25
 - D: GROUND SNOW LOAD (Pg): 25 PSF.
 IMPORTANCE FACTOR: 1.0
 Ce: 1.0
 - Ct: 1.0 Ct: 1.0 F. STAIRS: 100 PSF.
 - G. TRUCK LOAD: HS-20 AASHTO LOADING

3. WIND LOAD:

- A. BASIC WIND SPEED: V = 90 MPH
- B. EXPOSURE CATEGORY: C.
- C. INTERNAL PRESSURE COEFFICIENT: +0.18 & -0.18
- D. IMPORTANCE FACTOR: 1.0

4. SEISMIC LOAD:

- A. SPECTRAL ACCELERATION FOR A 1-SECOND: S1=0.052
 B. SPECTRAL ACCELERATION FOR SHORT PERIODS: Ss=0.163
- C. BUILDING OCCUPANCY CATEGORY: II
- D. SITE CLASSIFICATION: D
- E. IMPORTANCE FACTOR: 1.0 F. SPECTRAL RESPONSE ACCELERATION SD1=0.084
- G. SPECTRAL RESPONSE ACCELERATION SDs=0.174
- H. DESIGN CATEGORY: B



PROVIDED.

ASSUMPTIONS:

SLAB AND WALL: 6" MINIMUM REBAR SPACING WITH CONCRETE COVER = 1.5" CLEAR

BEAM: MINIMUM CLEAR SPACING BETWEEN BARS = 1.5db (BAR DIAMETER).
MINIMUM CONCRETE COVER = 1.5" CLEAR. MINIMUM STIRRUP #4@12"

TOP BAR: TOP BAR FOR SLAB AND BEAM SHALL BE DEFINED AS REINFORCEMENT SO PLACED THAT MORE THAN 12" OF CONCRETE IS CAST BELOW THE SPLICE

TENSION LAP SPLICE AND STANDARD HOOK LENGTH

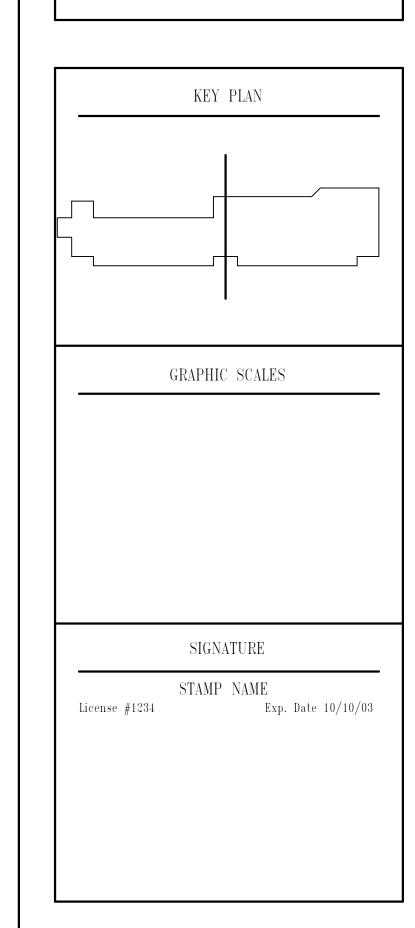
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95% SUBMISSION	05/16/08
CONSTRUCTABILITY REVIEW	10/01/08
BID SET	02/09/09



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HEALTH AND HUMAN SERVICES BUILDING





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GENERAL STRUCTURAL NOTES

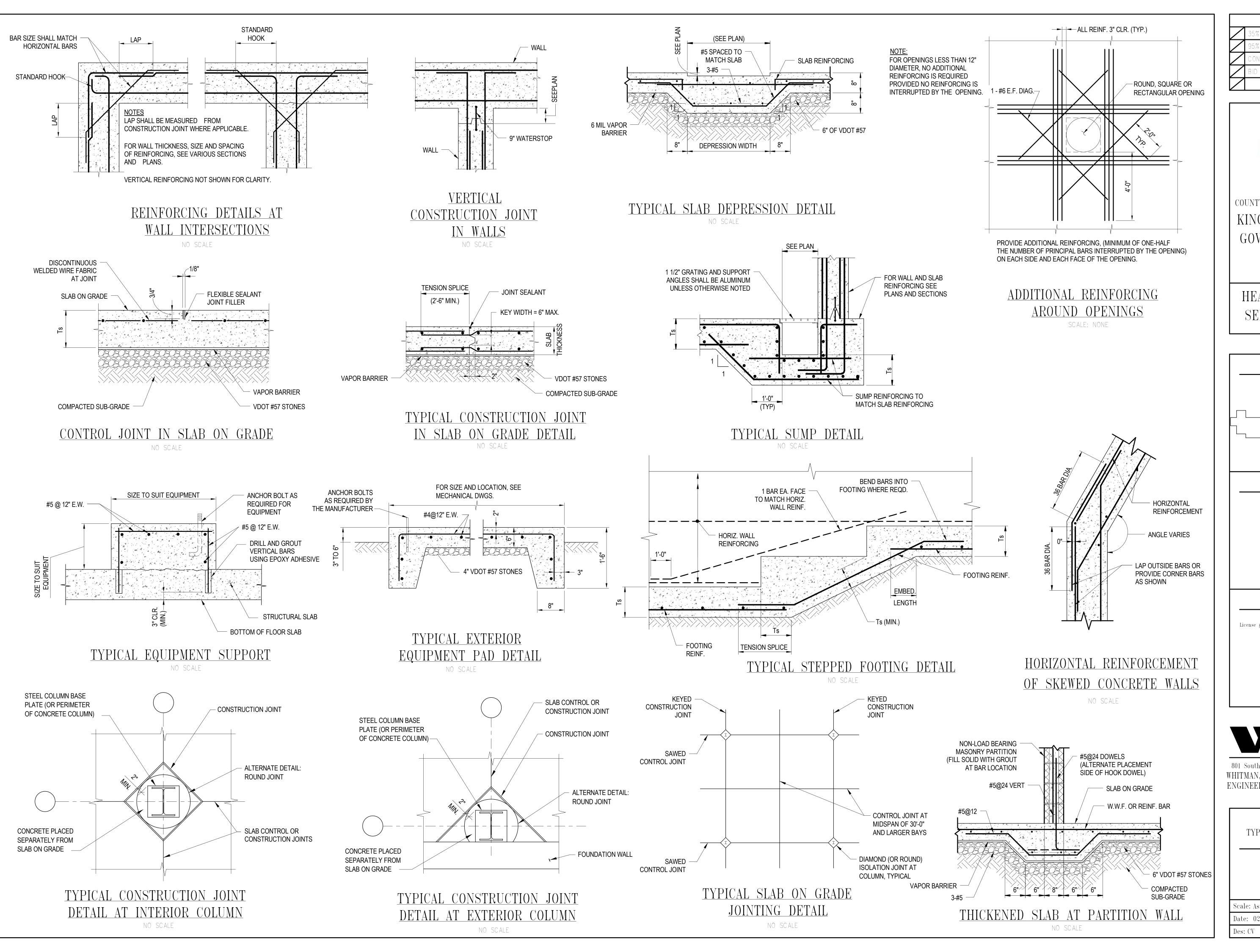
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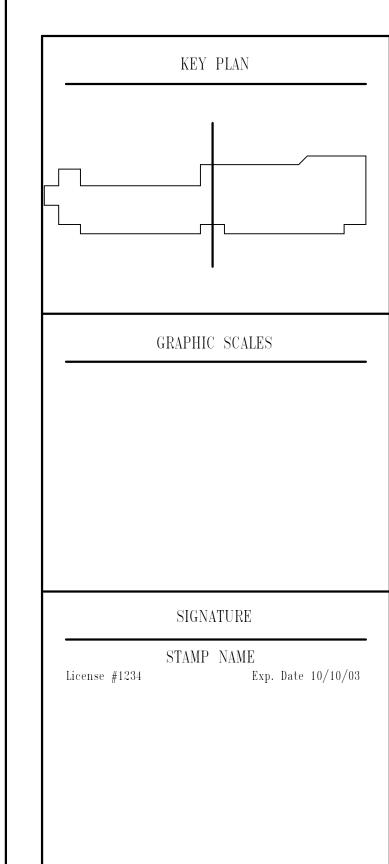
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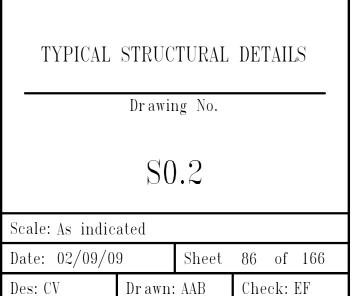
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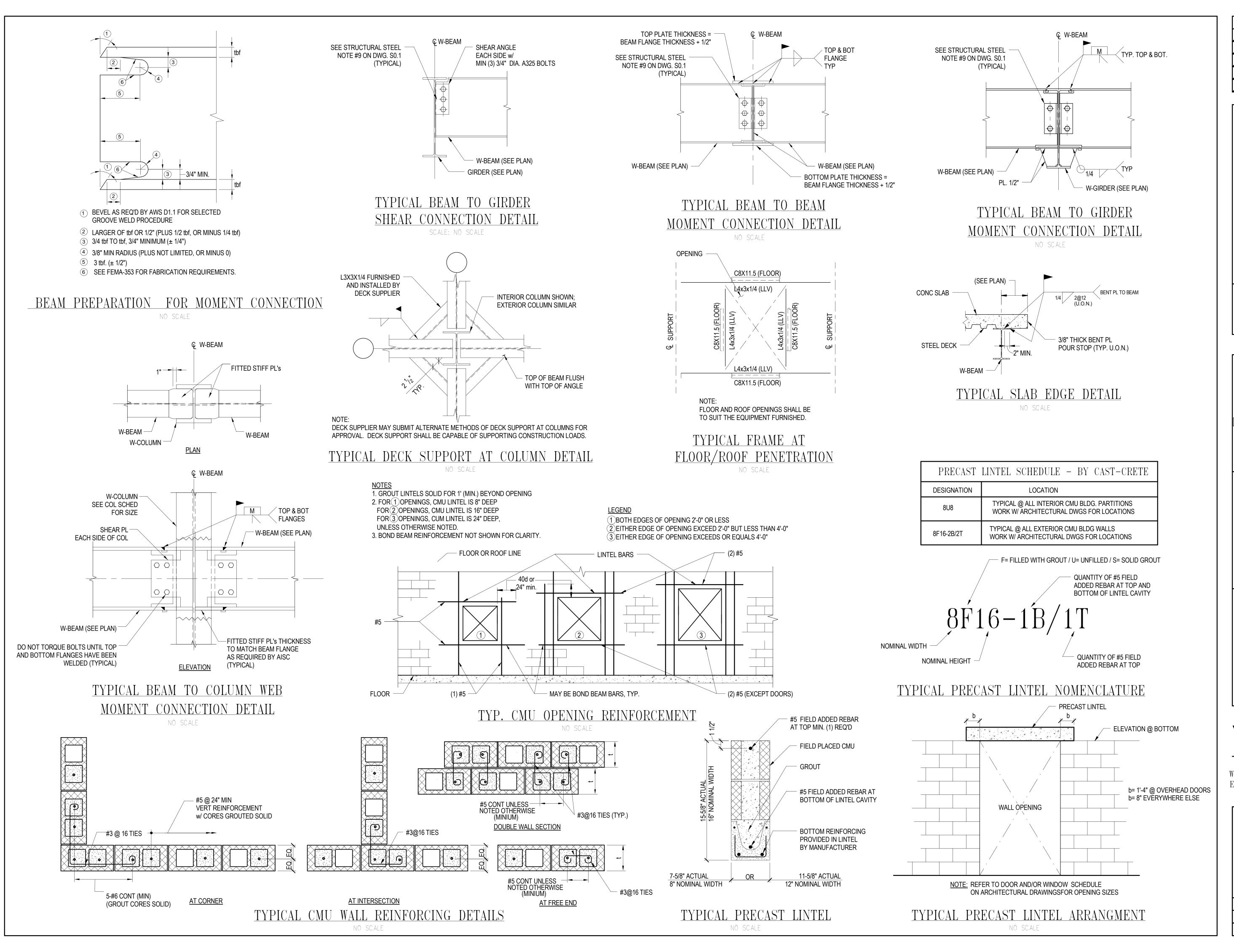
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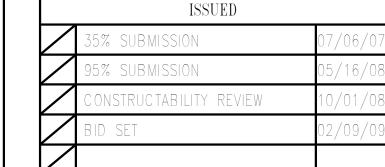
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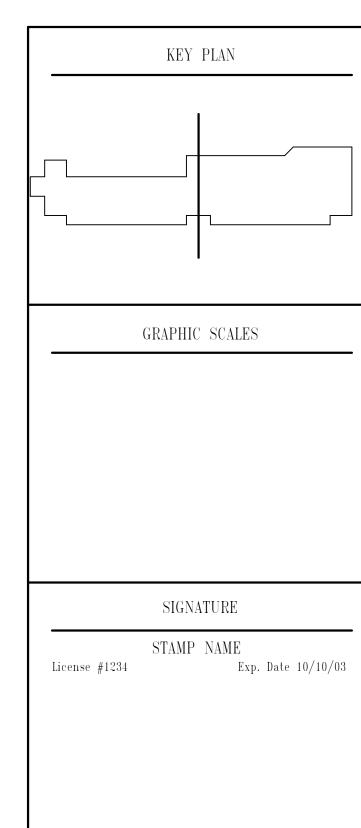




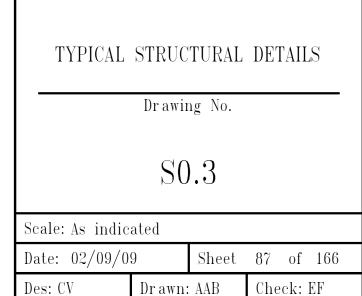


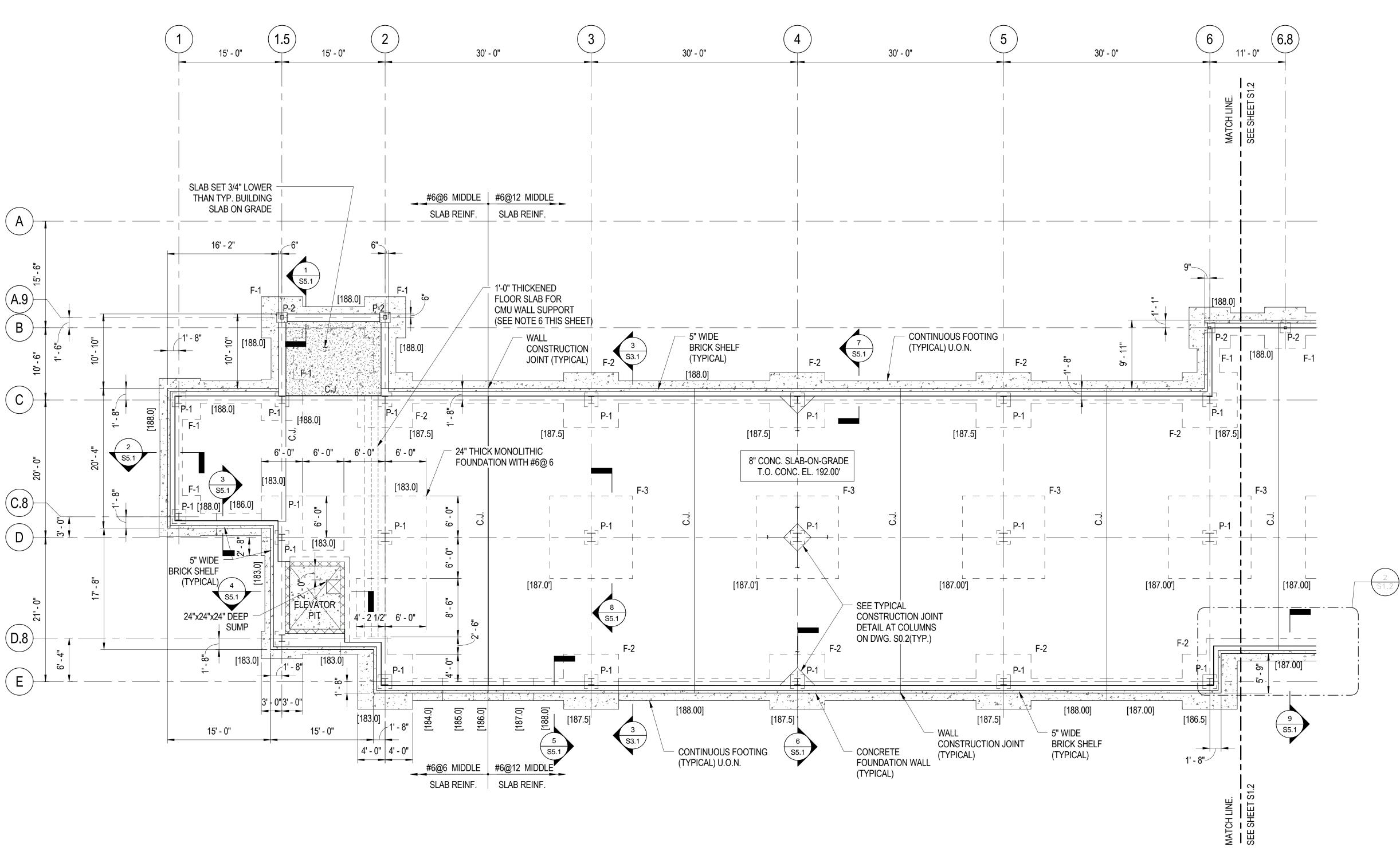
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FOUNDATION/FIRST FLOOR PLAN - PART A

LEGEND

[188.00'] INDICATES BOTTOM OF FOOTING/FOUNDATION ELEVATION.

F-1 INDICATES A FOOTING/FOUNDATION TYPE. SEE SHEET S6.1 FOR SCHEDULE.

INDICATES A PEDESTAL TYPE. SEE SHEET S6.1 FOR SCHEDULE.

C.J. INDICATES A SLAB AND/OR WALL CONSTRUCTION JOINT.

DRAWING NOTES

- 1. REFER TO SHEET A0.1 FOR ABBREVIATIONS.
- . REFER TO SHEET S0.1 FOR GENERAL STRUCTURAL NOTES.
- 3. REFER TO SHEET S0.2, AND S0.3 FOR TYPICAL STRUCTURAL DETAILS
- 4. FLOOR CONSTRUCTION TO BE 8" THICK REINFORCED CONCRETE SLAB-ON-GRADE, ON 6 MIL VAPOR BARRIER OVER 6-INCH LAYER OF VDOT #57 STONE. TOP OF CONCRETE FLOOR ELEVATION = 192.0
- REFER TO SHEET S6.1 FOR COLUMN SCHEDULE, PIER SCHEDULE, AND FOOTING SCHEDULE.
- REFER TO ARCHITECTURAL DRAWINGS FOR LOCATIONS OF CONCRETE MASONRY WALLS.

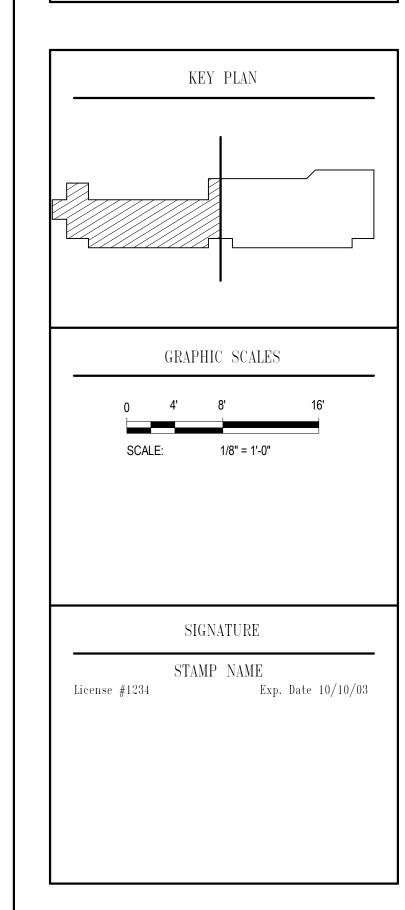
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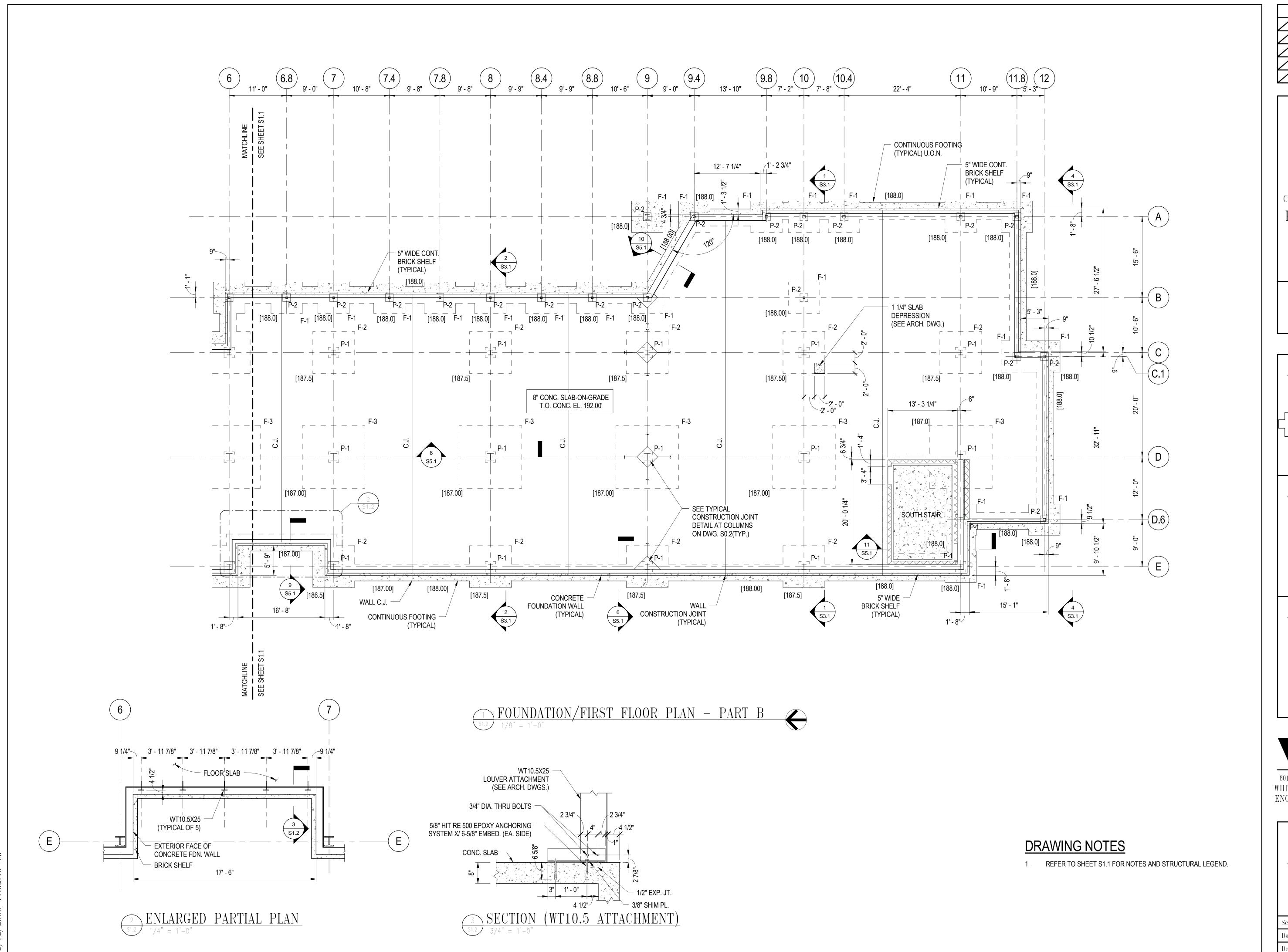




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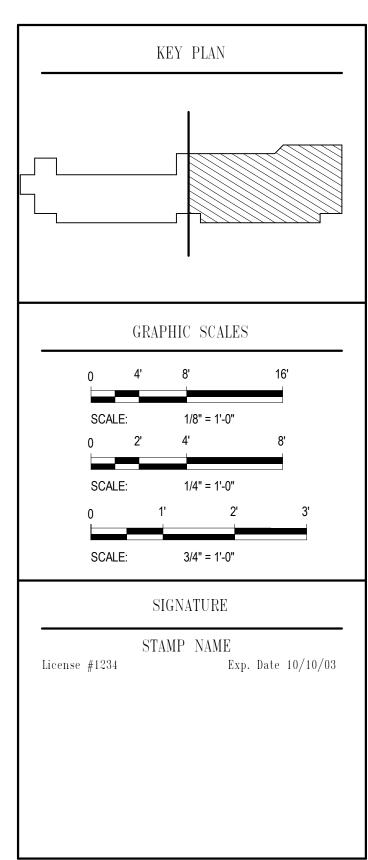


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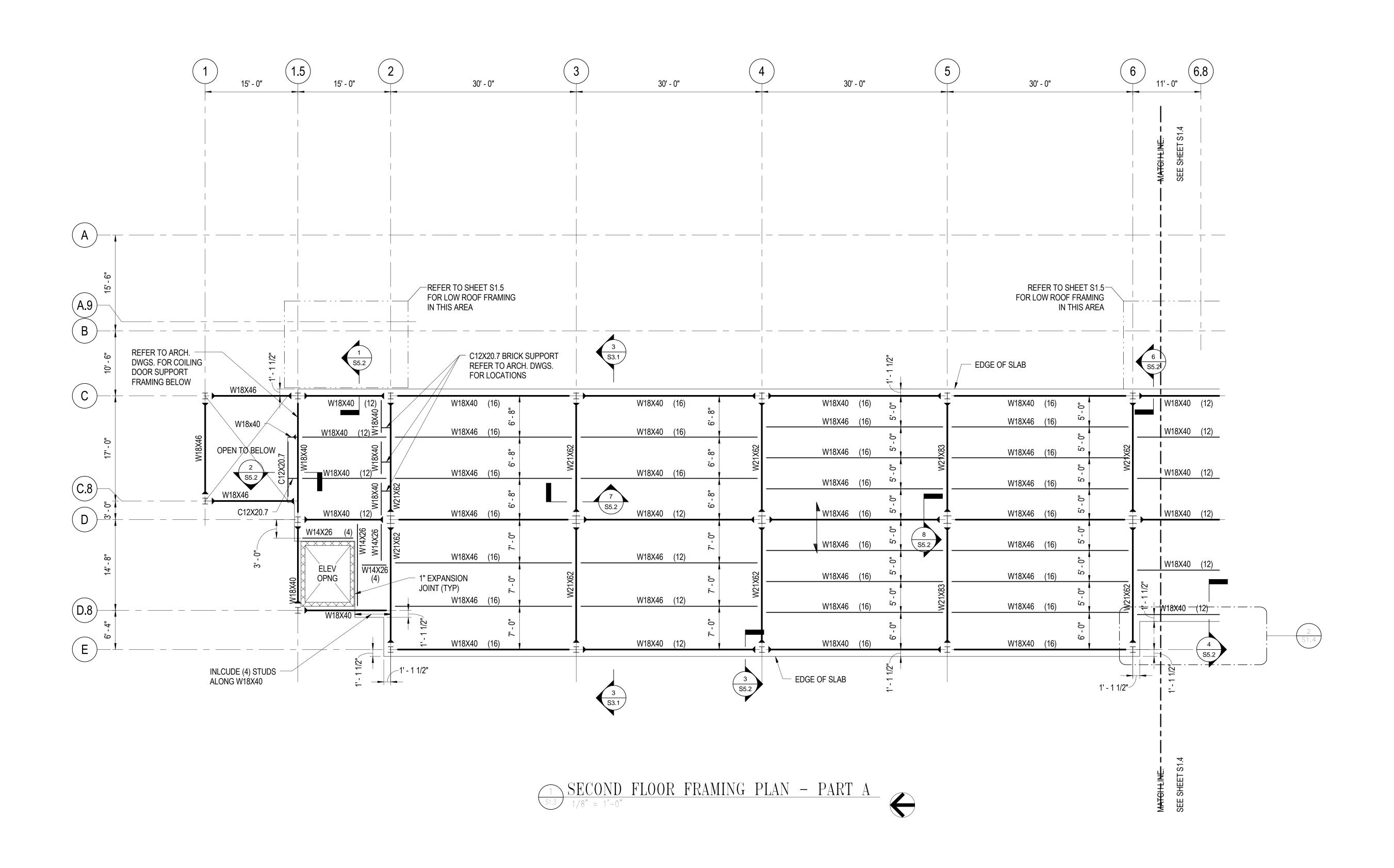
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FOUNDATION/FIRST FLOOR PLAN-PART B						
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STRUCTURAL LEGEND

INDICATES A MOMENT CONNECTION.

INDICATES A BEAM TO BEAM SHEAR CONNECTION.

DY26 (12) INDICATES A COMPOSITE STEEL REAM WITH THE REQUIRE

2X26 (12) INDICATES A COMPOSITE STEEL BEAM WITH THE REQUIRED NUMBER OF SHEAR CONNECTORS TO BE INSTALLED AT EQUAL SPACING ON THE BEAM.

INDICATES THE SPAN DIRECTION OF THE METAL DECK.

DRAWING NOTES

- 1. REFER TO SHEET S0.1 FOR GENERAL STRUCTURAL NOTES.
- 2. REFER TO SHEET S0.2, AND S0.3 FOR TYPICAL STRUCTURAL DETAILS
- 3. REFER TO ARCHITECTURAL DRAWINGS FOR THE LOCATIONS OF MASONRY WALLS AND WALL OPENINGS. PROVIDE DOWELS TO MATCH THE WALL REINFORCING SIZE AND SPACING.
 - REFER TO MECHANICAL DRAWINGS FOR THE LOCATIONS OF FLOOR OPENINGS AND EQUIPMENT PADS.
- 5. TOP OF STEEL BEAM ELEVATION = 205.54' UNLESS OTHERWISE NOTED THUS (+/-).
- 5-1/2" THICK COMPOSITE FLOOR CONSTRUCTION CONSIST OF 2", 18 GAUGE STEEL DECK, WITH A 3 1/2" CONCRETE TOPPING, REINFORCED WITH $6x6-W2.9 \times W2.9 \text{ WELDED WIRE FABRIC}$, POSITIONED 1" BELOW THE TOP OF THE SLAB. PROVIDE 3/4" DIAMETER x 41/2" LONG, HEADED STUDS, SPACED EQUALLY ON BEAMS INDICATED THUS (16), ON THE FLOOR FRAMING PLAN. TOP OF CONCRETE FINISH FLOOR ELEVATION = 206.00'

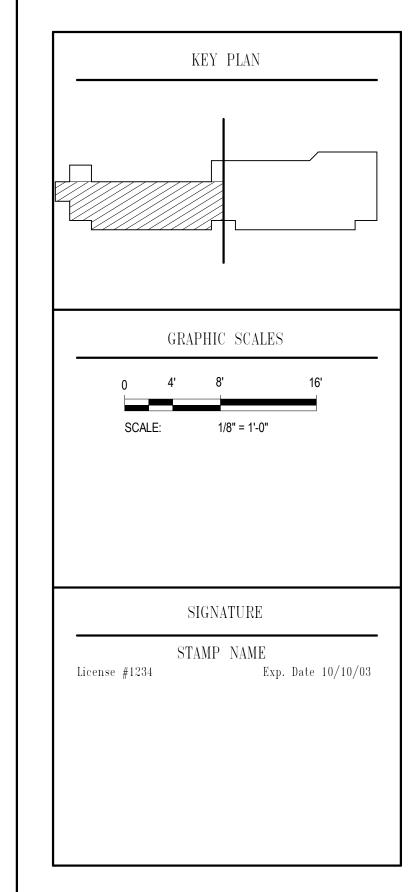
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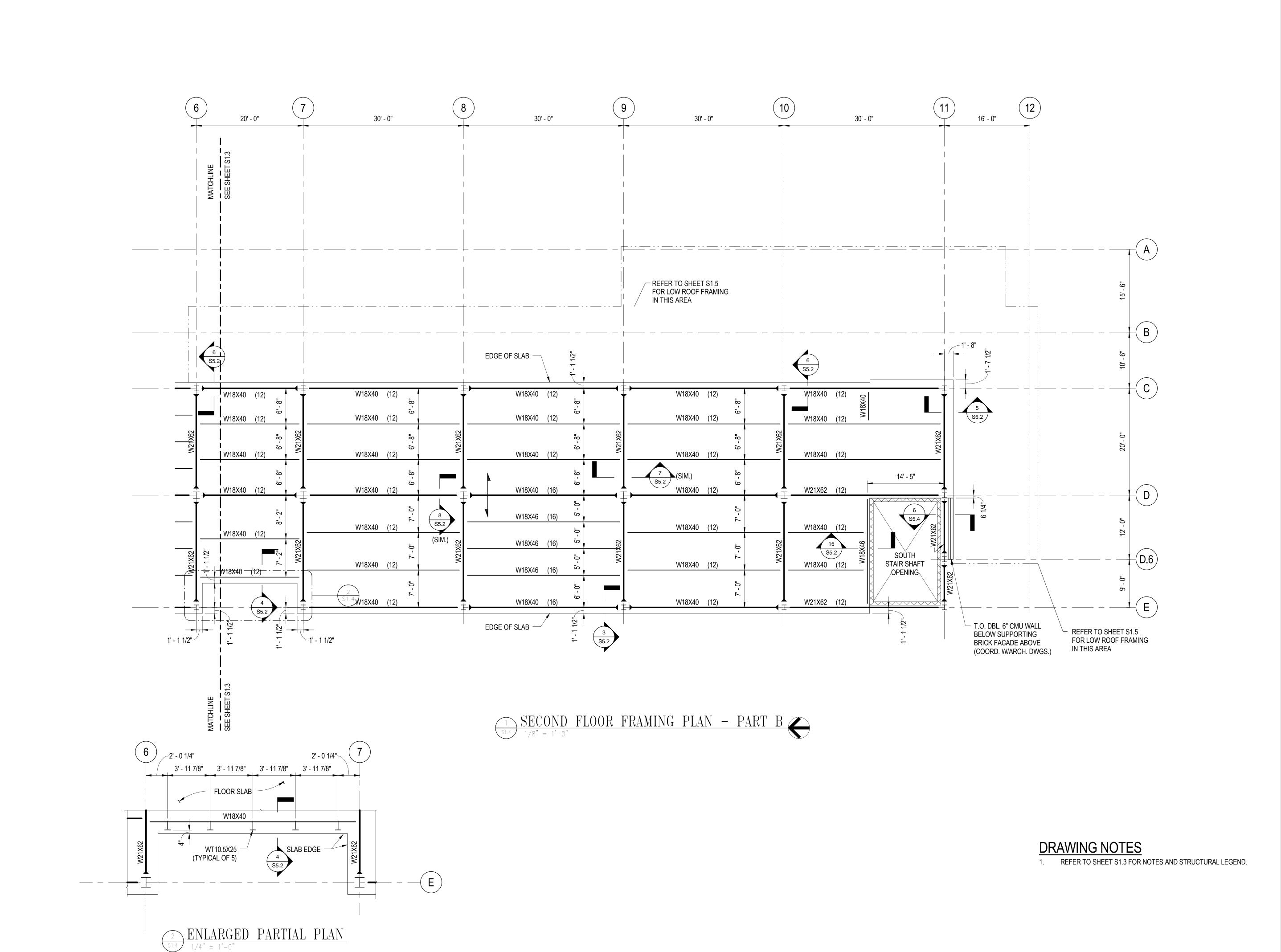


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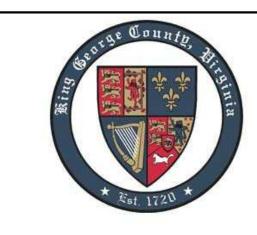
SECOND FLOOR FRAMING PLAN-PART A

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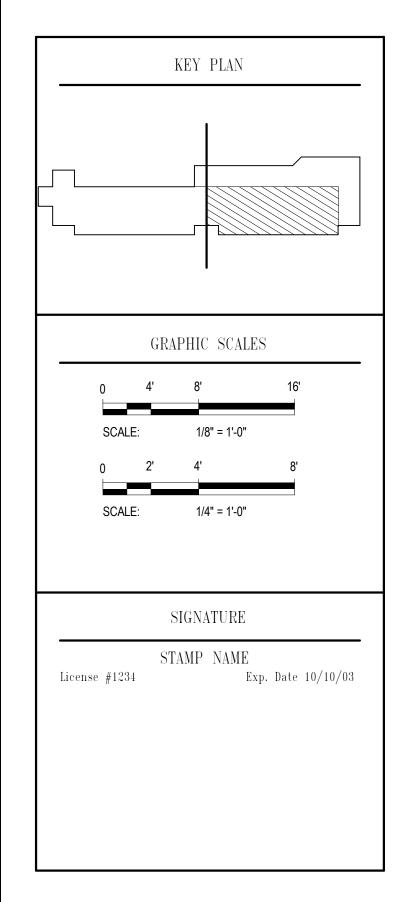


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SECOND FLOOR FRAMING
PLAN-PART B

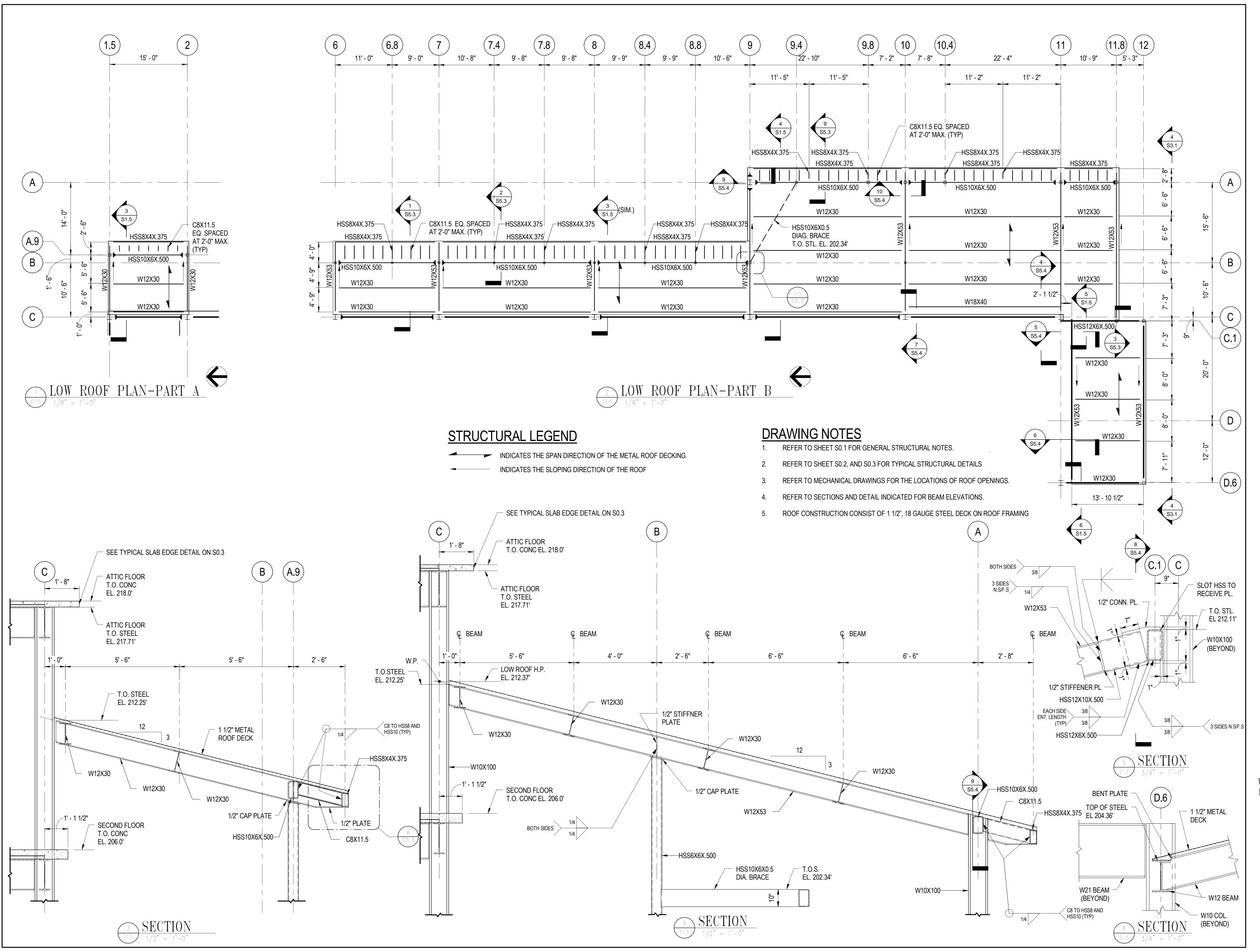
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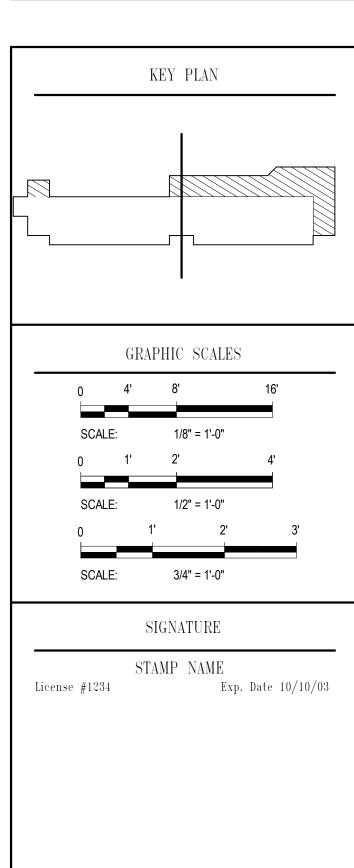


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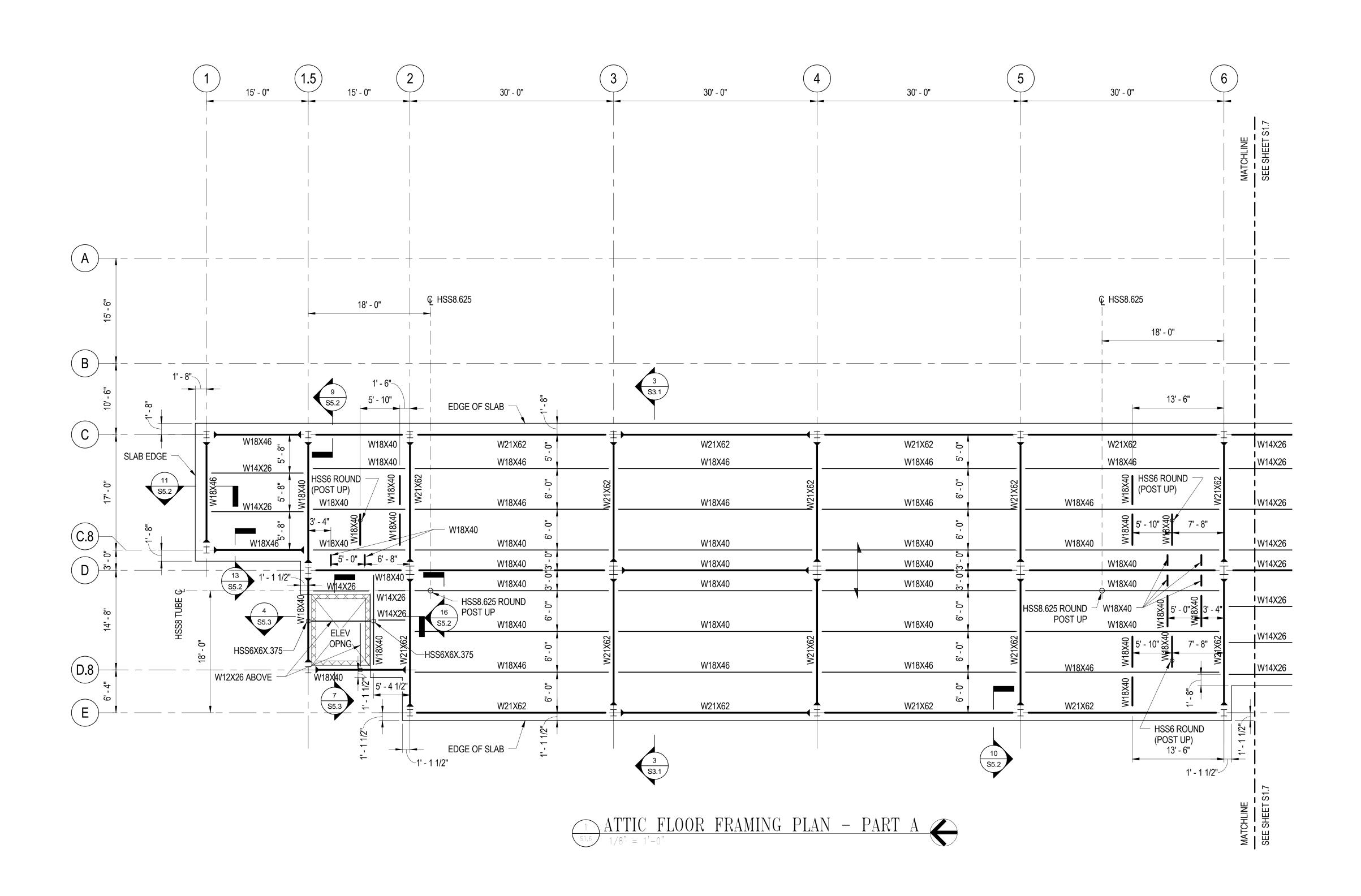
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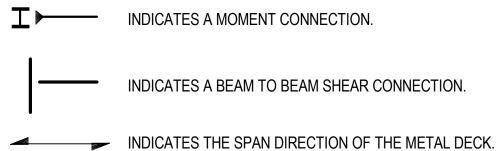




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STRUCTURAL LEGEND



DRAWING NOTES

- REFER TO SHEET S0.1 FOR GENERAL STRUCTURAL NOTES.
- 2. REFER TO SHEET S0.2, AND S0.3 FOR TYPICAL STRUCTURAL DETAILS
- 4. REFER TO MECHANICAL DRAWINGS FOR THE LOCATIONS OF FLOOR OPENINGS.
- 5. TOP OF STEEL BEAM ELEVATION = 217.71' UNLESS OTHERWISE NOTED THUS (+/-).
- 6. FLOOR CONSTRUCTION CONSIST OF 1-1/2", 18 GAUGE STEEL DECK, WITH A 2" CONCRETE TOPPING (TOTAL 3 1/2" THICK) REINFORCED WITH 6x6-W2.9 x W2.9 WELDED WIRE FABRIC POSITIONED 1" BELOW THE TOP OF THE SLAB. TOP OF CONCRETE FINISH FLOOR ELEVATION = 218.00'

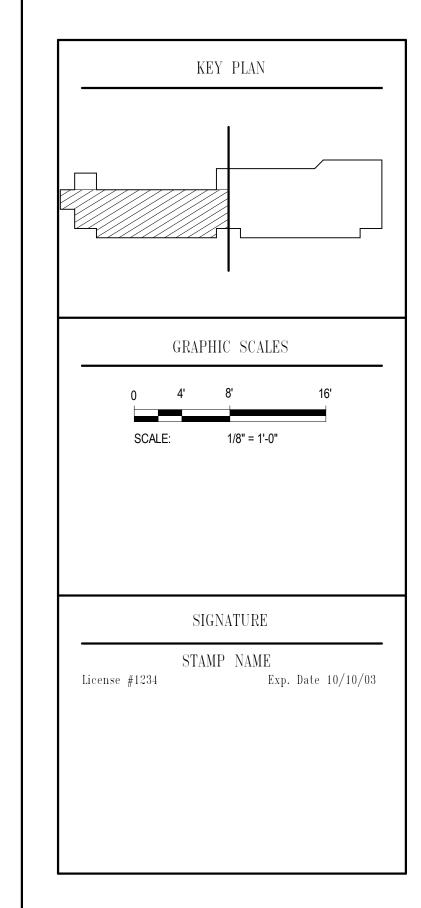
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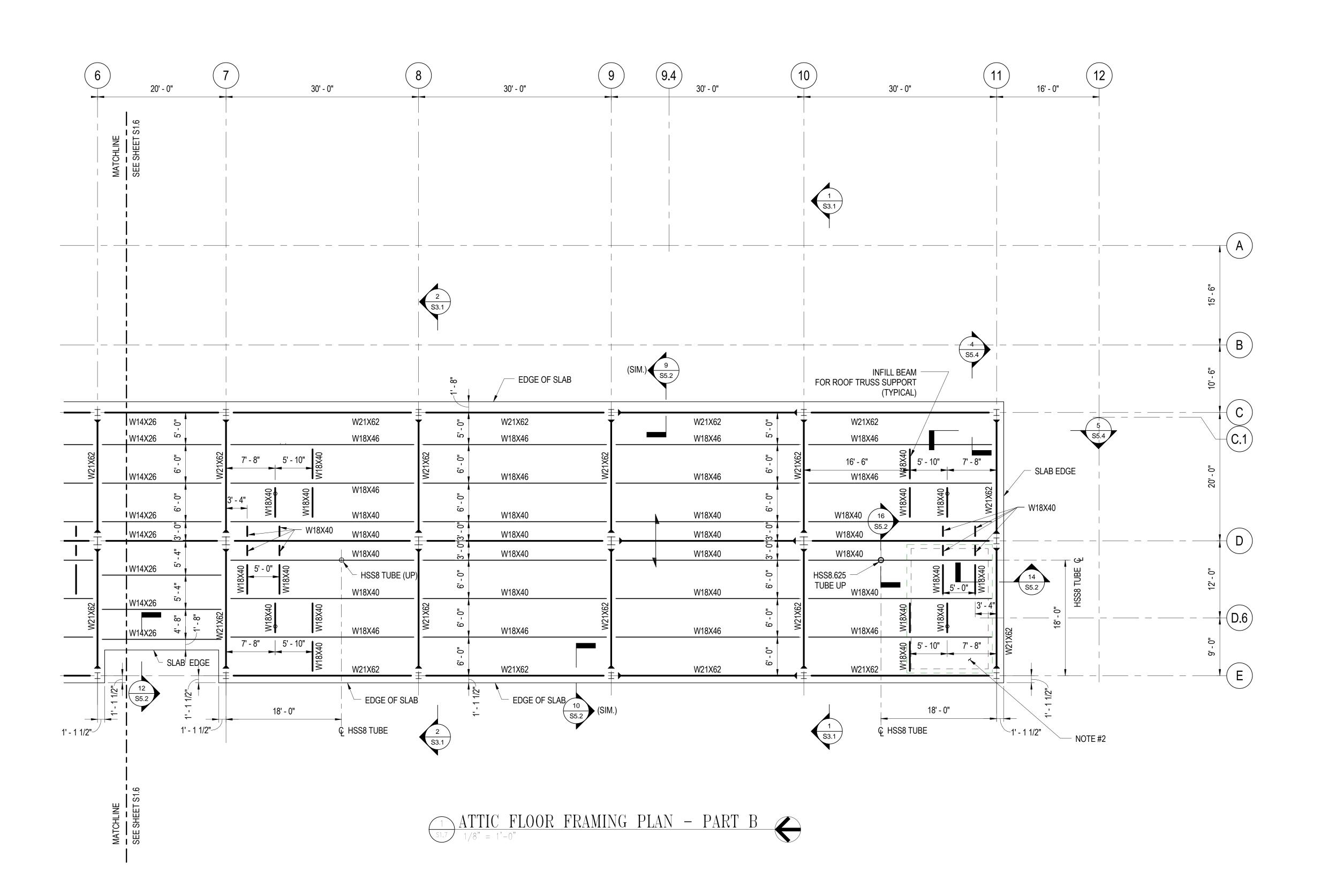
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<u>NOTES</u>

- 1. REFER TO SHEET S1.6 FOR NOTES AND STRUCTURAL LEGEND.
- 2. 6-INCH THICK CONCRETE COVER SLAB WITH #5@12 MIDDLE, EACH WAY OVER SOUTH STAIR SHAFT. TOP OF CONCRETE ELEVATION = 216.00'

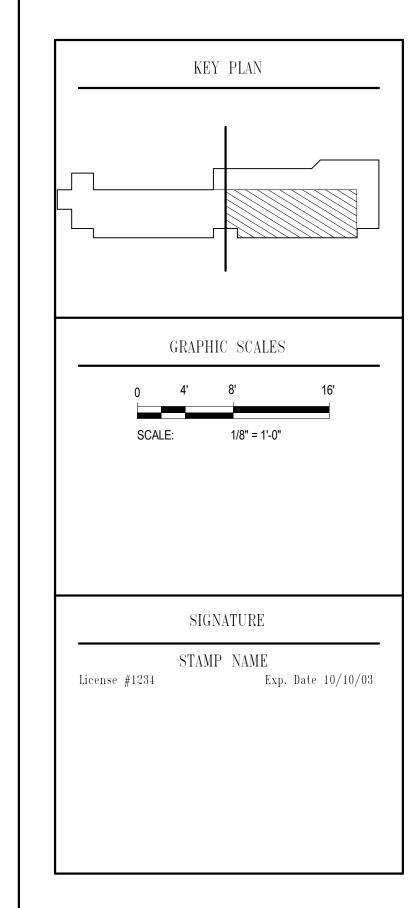
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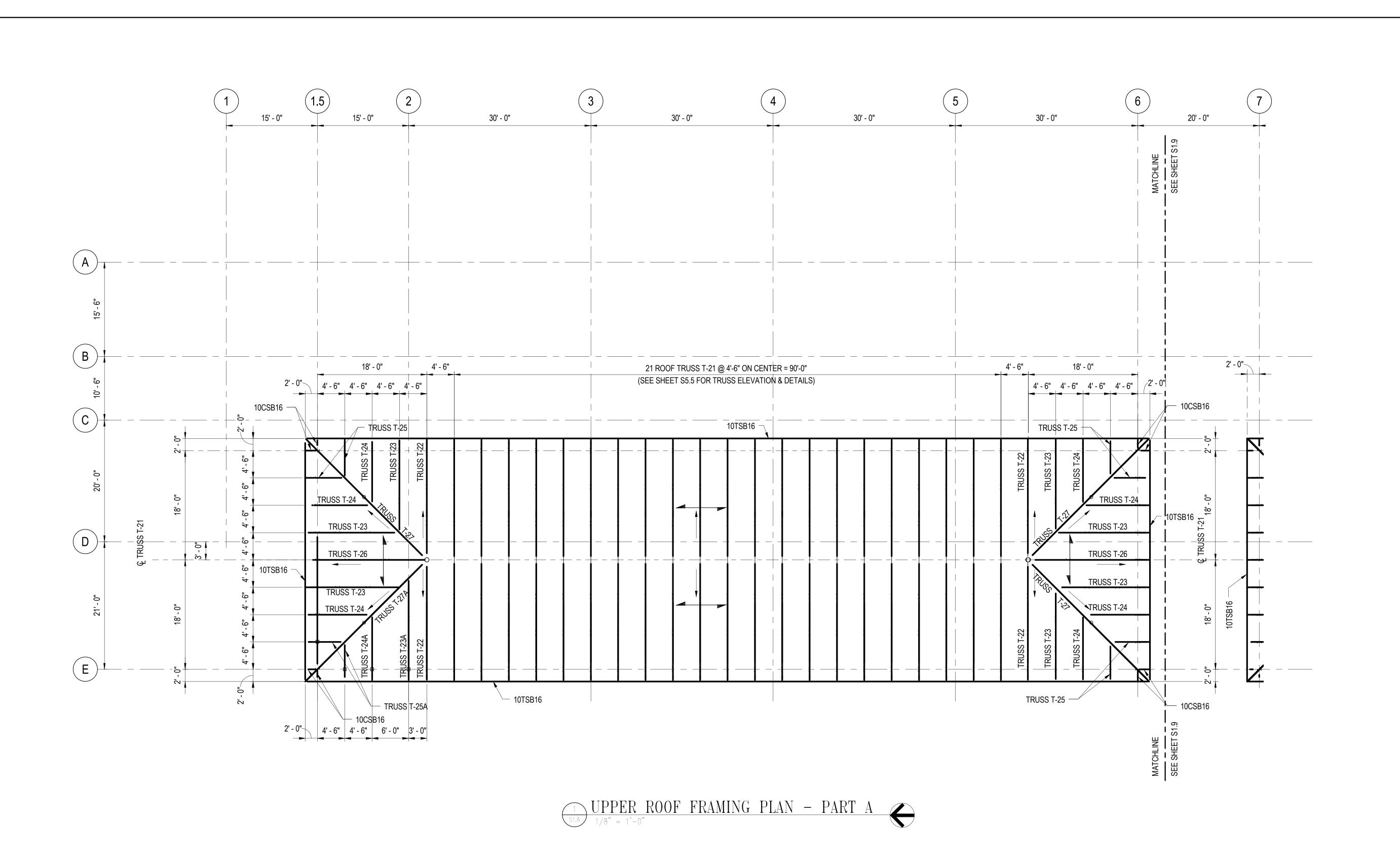
ATTIC FLOOR FRAMING
PLAN-PART B

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DRAWING NOTES

- 1. REFER TO SHEET S0.1 FOR GENERAL STRUCTURAL NOTES.
- 2. REFER TO SHEET S0.2, AND S0.3 FOR TYPICAL STRUCTURAL DETAILS
- 4. REFER TO MECHANICAL DRAWINGS FOR THE LOCATIONS OF ROOF OPENINGS.
- 5. REFER TO SHEETS S5.5, S5.6, S5.7 AND S5.8 FOR ROOF TRUSS ELEVATIONS AND DETAILS.
- 6. ROOF CONSTRUCTION CONSIST OF 1-1/2", 18 GAGE STEEL DECK, ON LIGHT-GAUGE ROOF TRUSS. REFER TO ARCHITECTURAL DRAWINGS FOR ADDITIONAL DETAILS OF ROOF CONSTRUCTION.
- 7. INDICATES THE SPAN DIRECTION OF THE STEEL ROOF DECK.

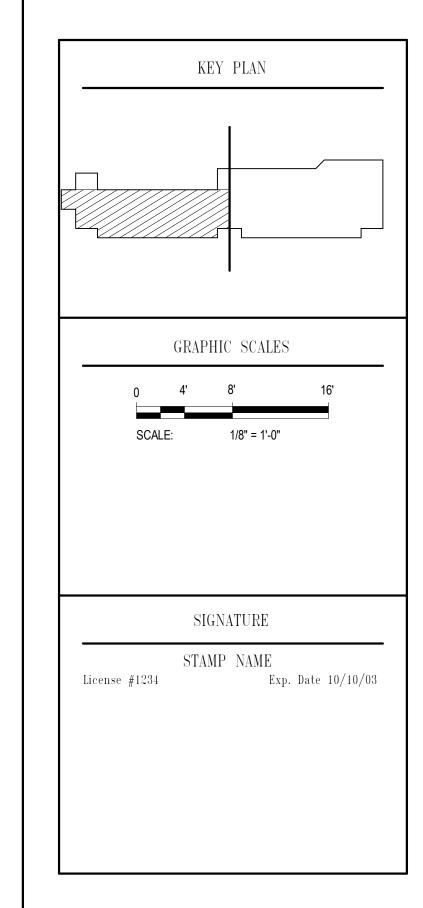
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UPPER ROOF FRAMING
PLAN-PART A

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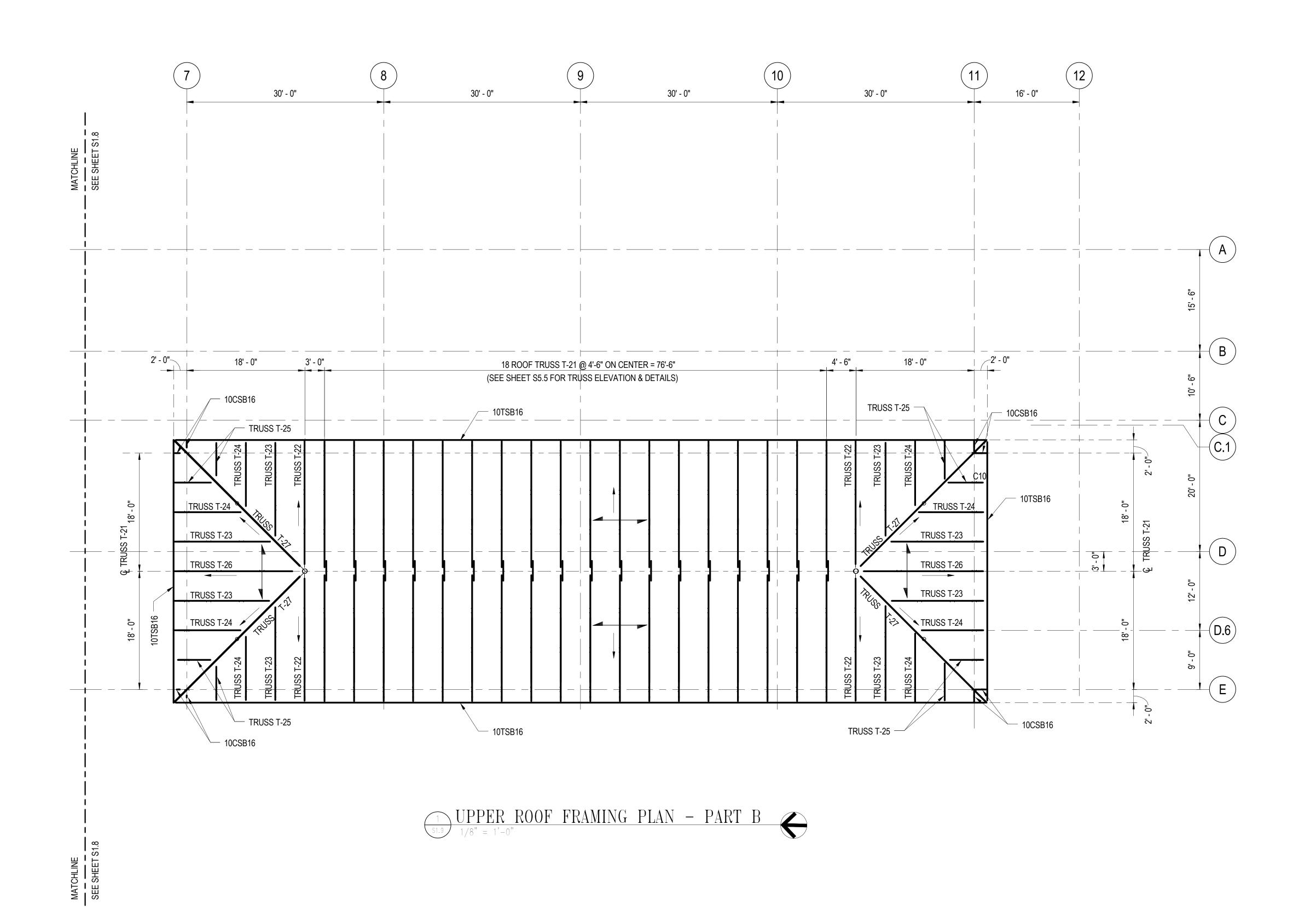
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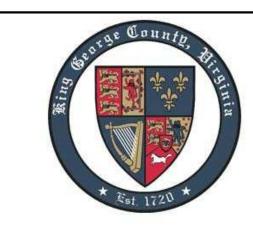
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DRAWING NOTES

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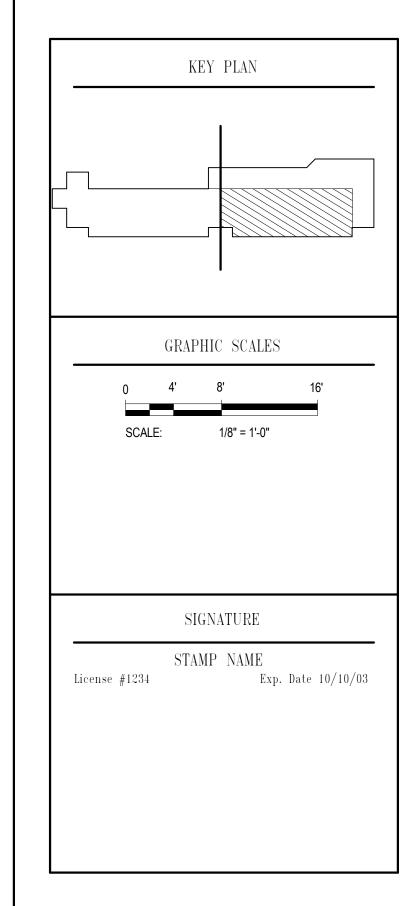
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UPPER ROOF FRAMING
PLAN-PART B

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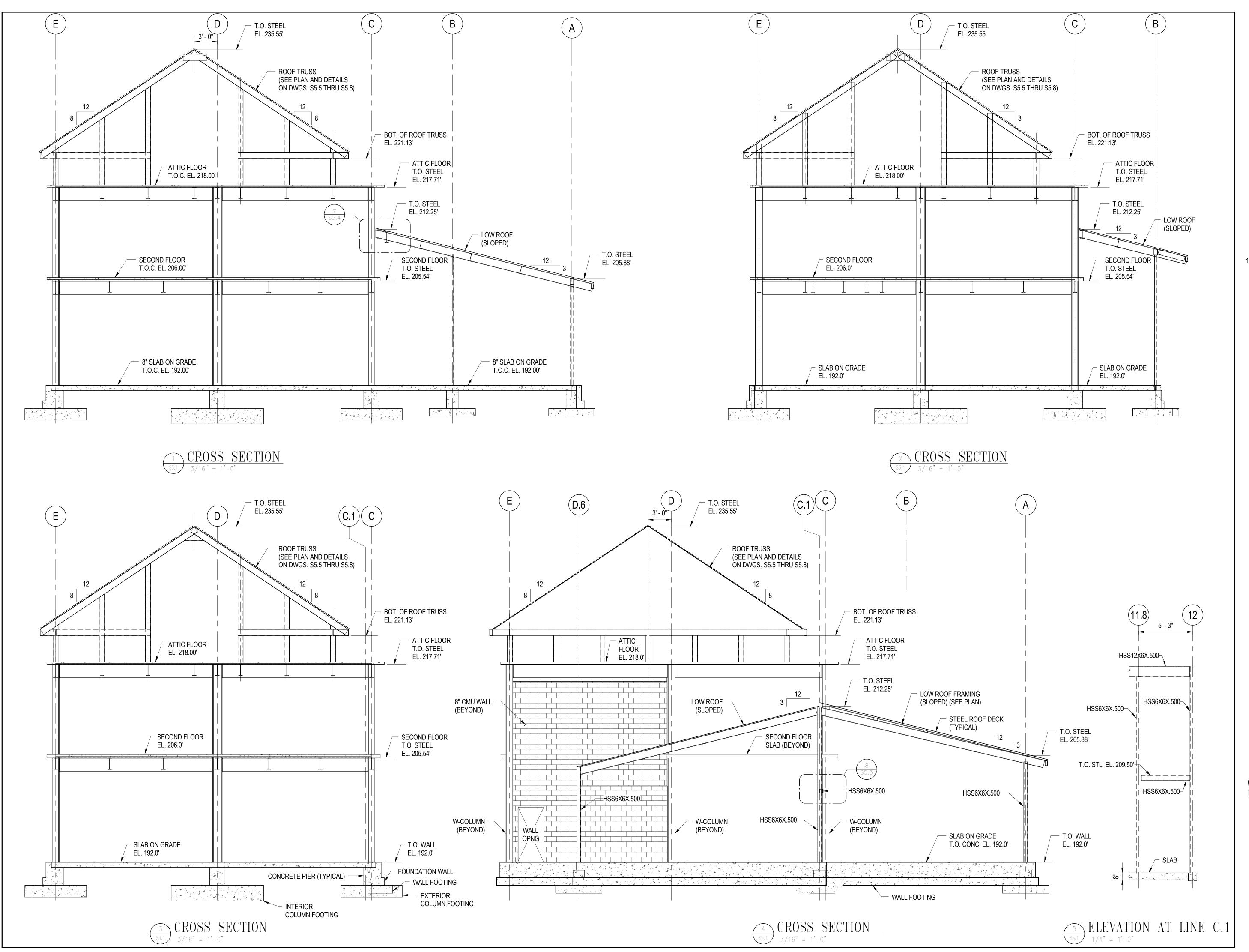
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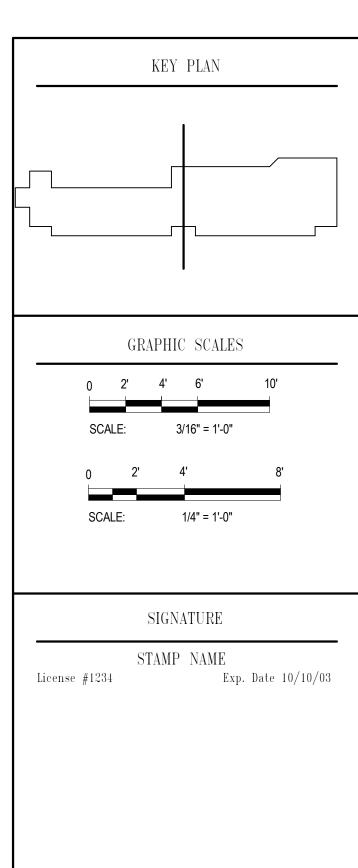
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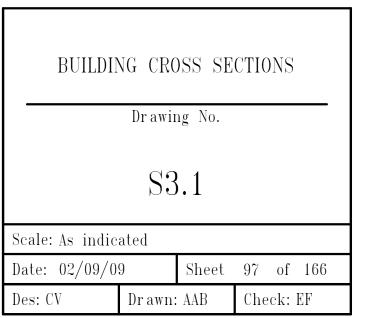
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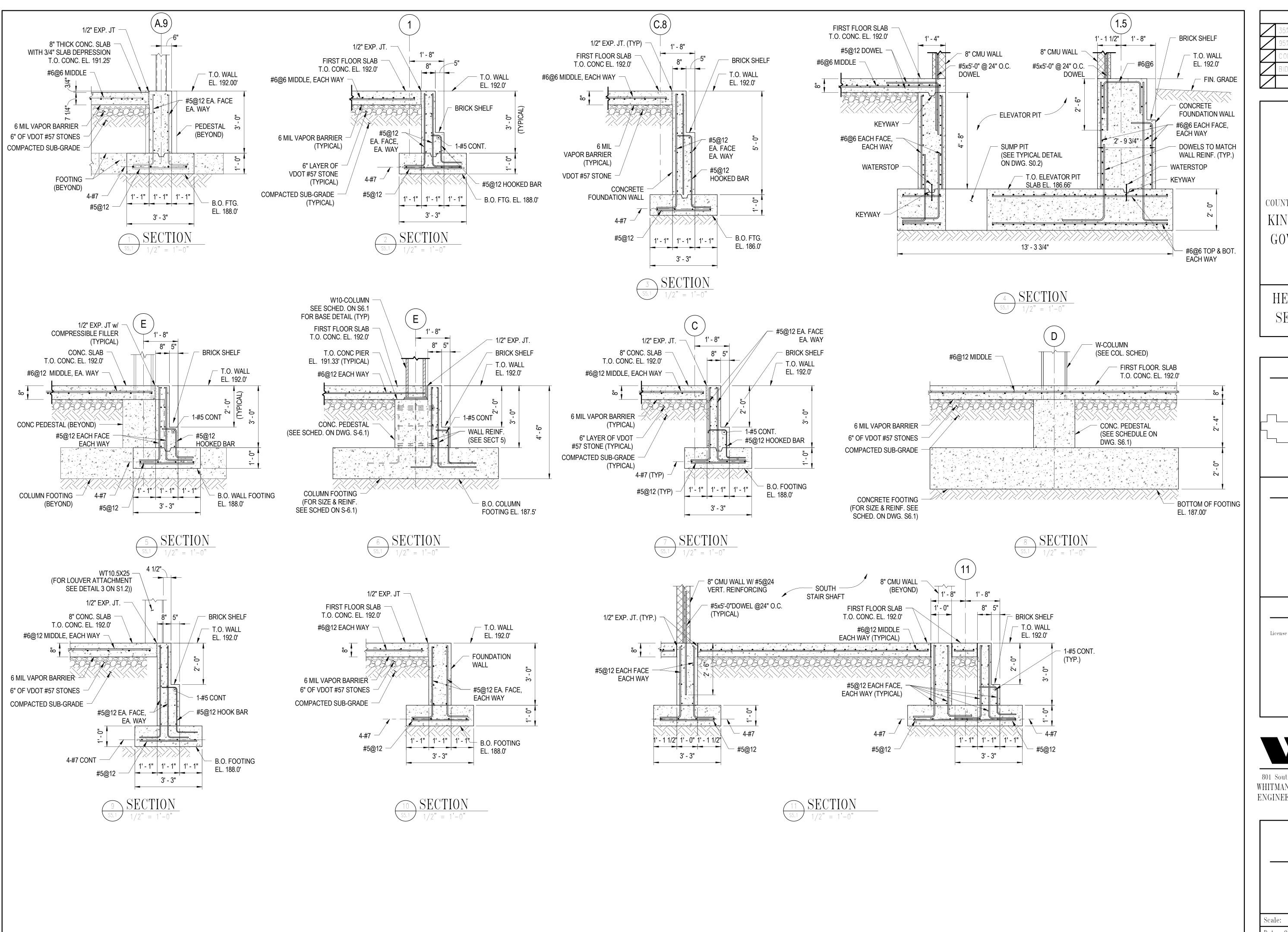
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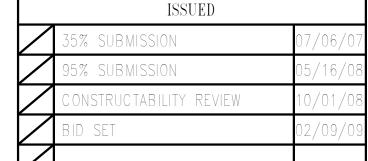
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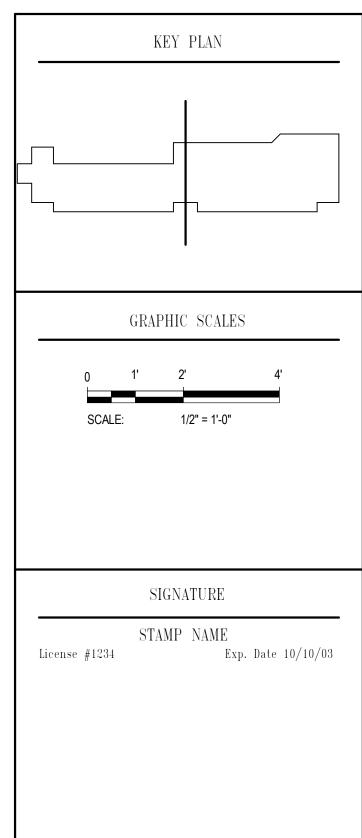




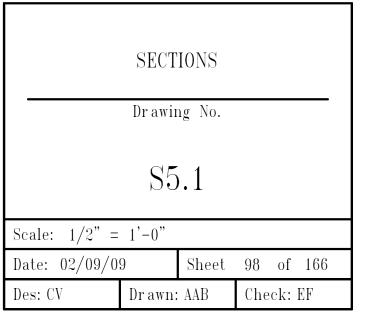


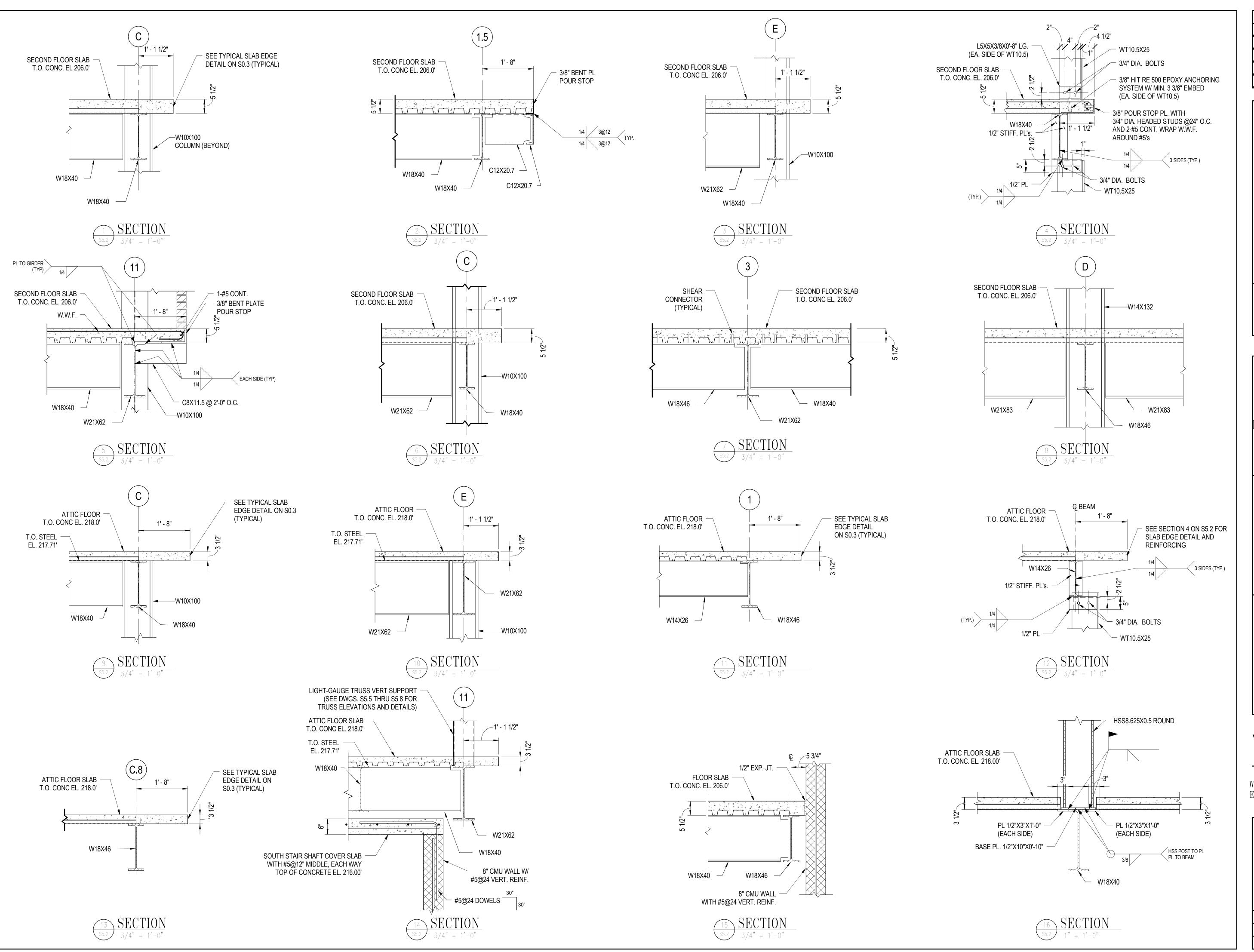
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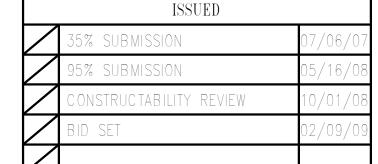
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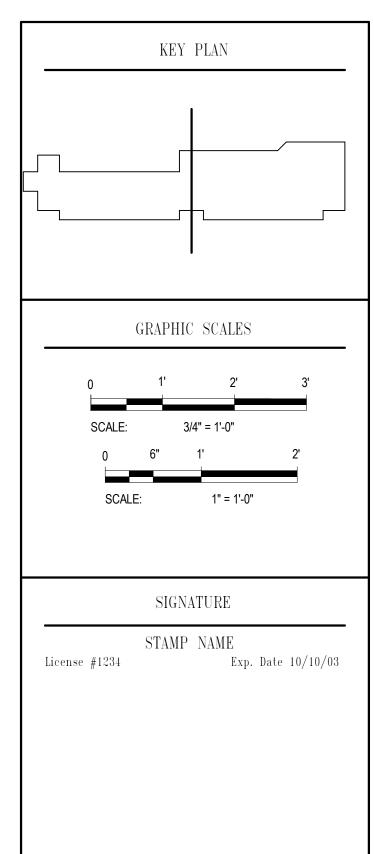




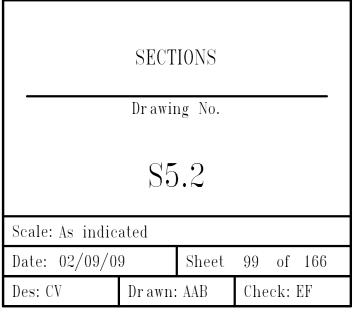


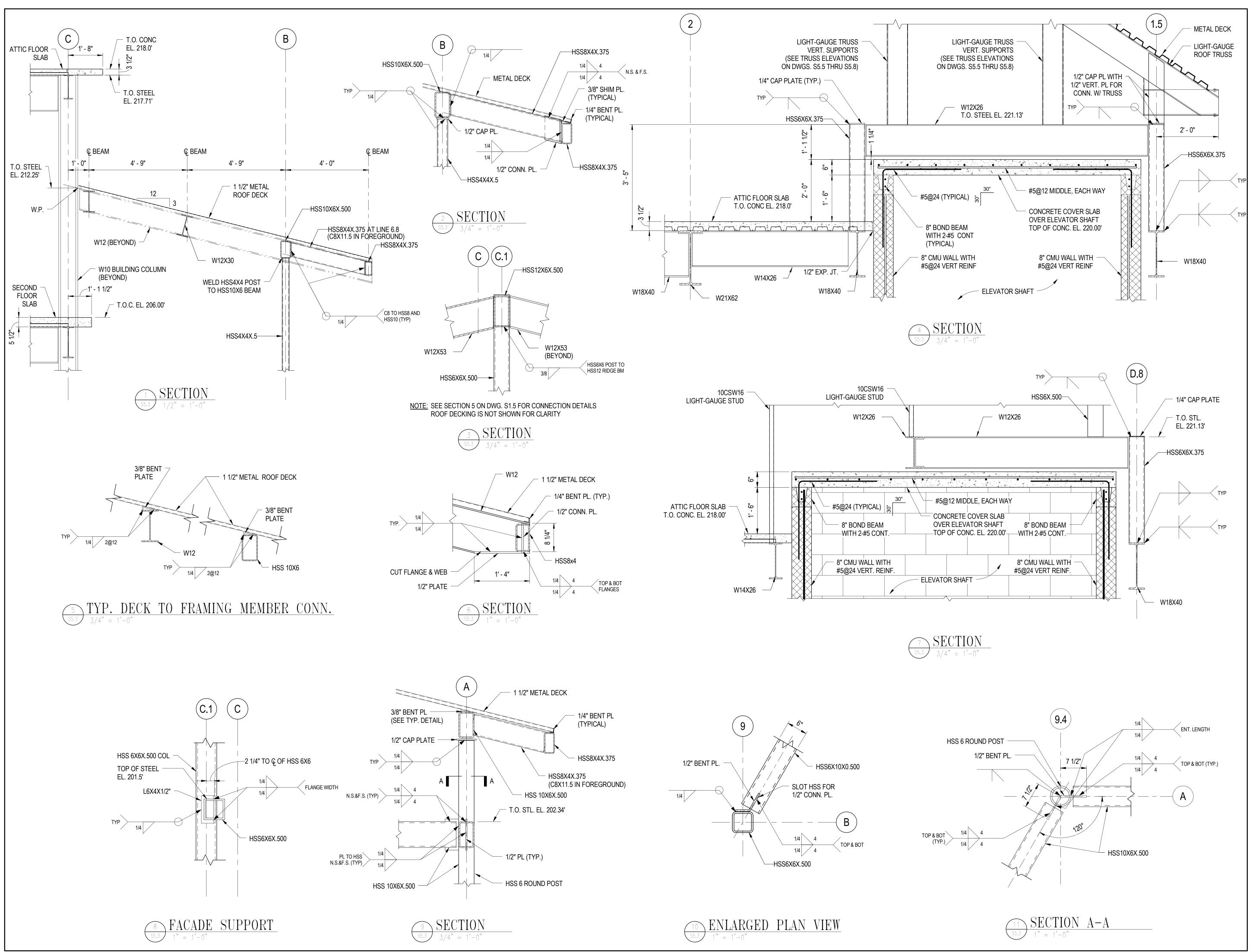
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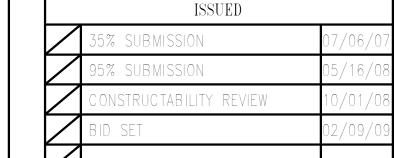
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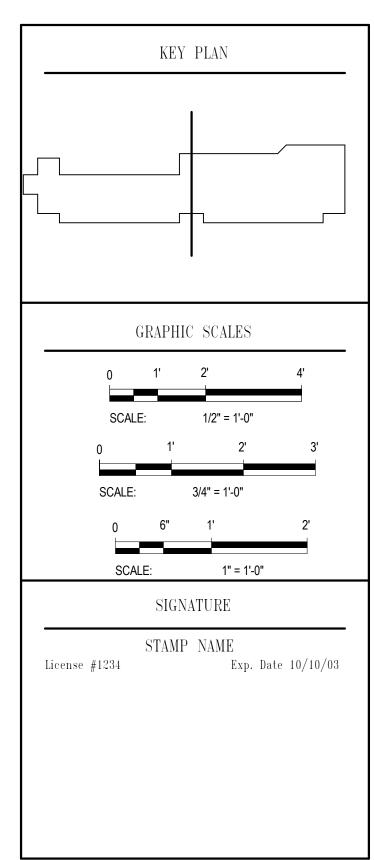






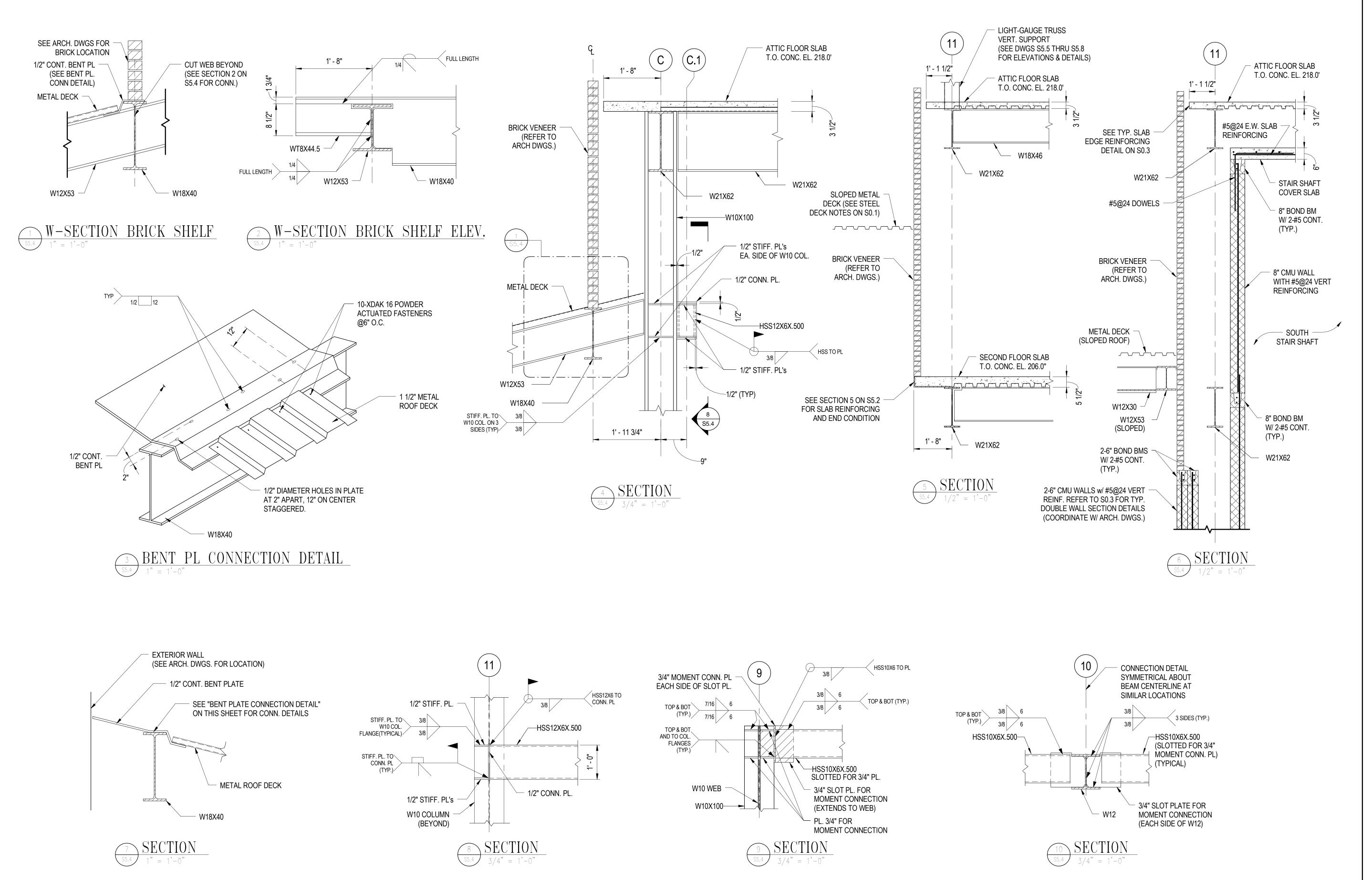
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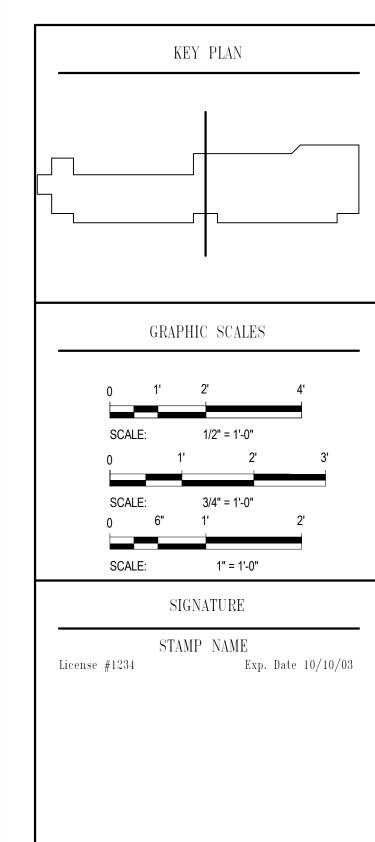


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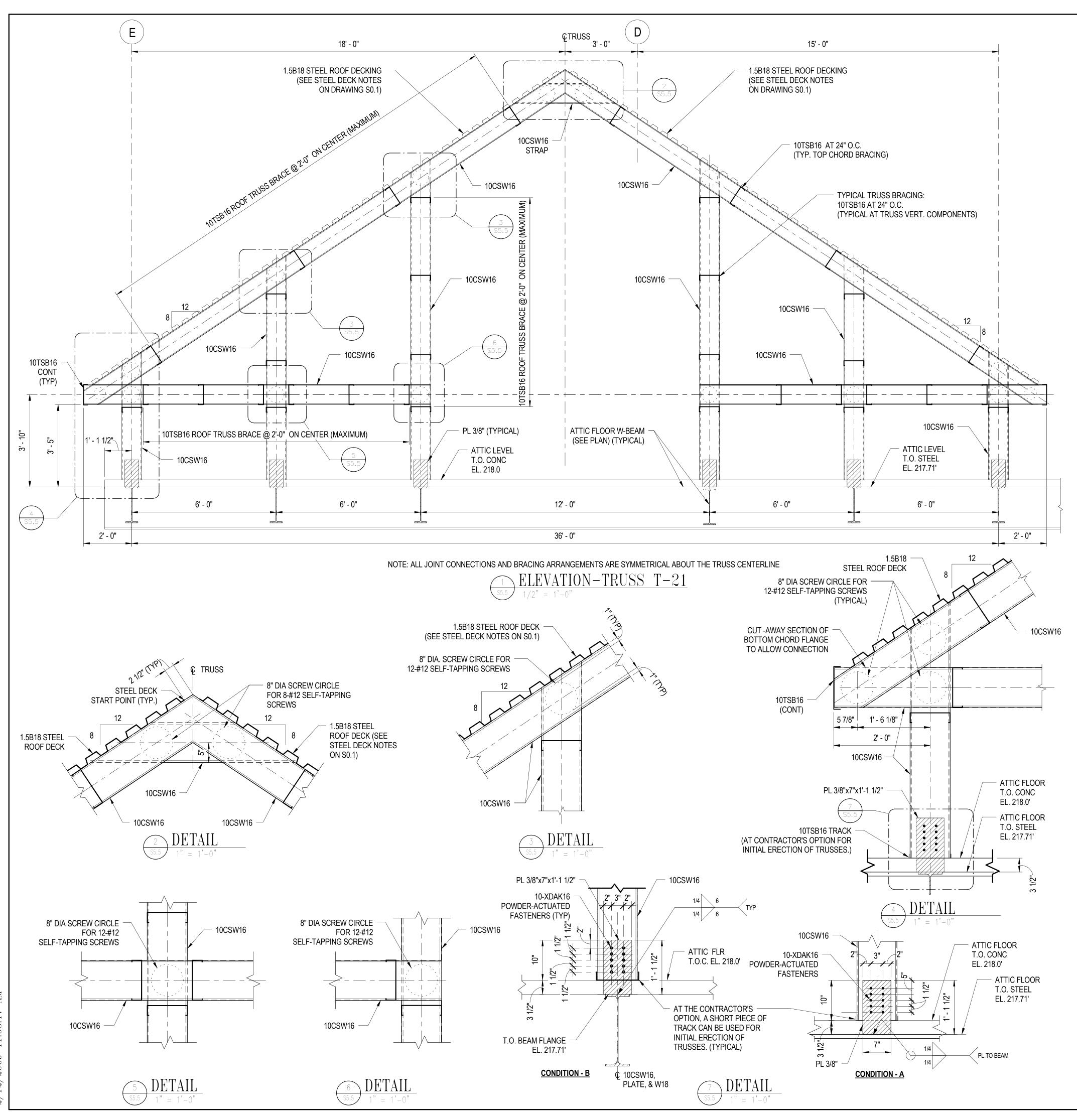
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LIGHT-GAUGE NOTES

A. DESIGN CRIT

 AMERICAN IRON AND STEEL INSTITUTE CODE, "MANUAL OF COLD FORMED STEEL DESIGN," LATEST EDITION.

2. AWS, D1.1 AND D1.3, LATEST EDITION

B. GENERAL LIGHT- GAUGE NOTES

- 1. THE LIGHT- GAUGE SUB-CONTRACTOR SHALL VERIFY ALL NOTES AND DIMENSIONS PRIOR TO FABRICATION, OR START OF CONSTRUCTION.
- 2. THE LIGHT- GAUGE SUB-CONTRACTOR SHALL PROVIDE ALL MEASURES NECESSARY TO PROTECT THE STRUCTURE, THE WORK PERSONS AND OTHERS DURING CONSTRUCTION. THE SUB-CONTRACTOR SHALL SUPERVISE AND DIRECT THE WORK AND BE RESPONSIBLE FOR ALL LIGHT-GAUGE CONSTRUCTION.
- 3. NO STRUCTURAL MEMBER SHALL BE CUT NOTCHED, OR OTHERWISE BE REDUCED IN STRENGTH.
- THE LIGHT- GAUGE SUB-CONTRACTOR SHALL COORDINATE ARCHITECTURAL, MECHANICAL, AND ELECTRICAL DRAWINGS FOR ANCHORED, EMBEDDED, AND/OR SUPPORTED ITEMS OR ANY OTHER WHICH MAY AFFECT THE STRUCTURAL WORK.
- THE LIGHT- GAUGE SUB-CONTRACTOR SHALL SUBMIT TO THE STRUCTURAL ENGINEER FOR REVIEW, THREE COPIES OF THE MANUFACTURER'S SPECIFICATIONS, AND LOAD TESTS FOR ALL FASTENERS. PRIOR TO STARTING ANY LIGHT- GAUGE CONSTRUCTION WORK.

C. LIGHT- GAUGE METAL FRAMING

(a) STANDARDS

- 1. ASTM A-653 SS50, CLASS 1, WITH A MINIMUM YIELD OF 50,000 PSI STEEL SHEET, ZINC-COATED (GALVANIZED) BY THE HOT-DIP PROCESS, STRUCTURAL (PHYSICAL) QUALITY.
- 2. LIGHT- GAUGE MANUFACTURER SHALL BE "DIETRICH INDUSTRIES, INC." NO SUBSTITUTIONS ARE ALLOWED WITHOUT THE STRUCTURAL ENGINEER'S APPROVAL. CONTRACTOR MUST USE THE LIGHT- GAUGE ELEMENTS CITED IN THESE DOCUMENTS. THE STRUCTURAL DESIGN IS DEPENDENT ON THE PHYSICAL PROPERTIES AND GENERAL CONFIGURATION OF THESE ELEMENTS AS OUTLINED IN THE MANUFACTURER'S PUBLISHED LITERATURE.
- 3. TRUSS BRACING (BRIDGING) IS REQUIRED TO SUPPORT THE TRUSSES AGAINST LATERAL MOVEMENT DURING CONSTRUCTION, AND TO HOLD THE TRUSSES IN THE LOCATIONS AS SHOWN ON THE PLANS. THE BRACING SHALL CONSIST OF A CONTINUOUS HORIZONTAL LIGHT-GAUGE MEMBER CONNECTING THE TRUSSES AS INDICATED ON THE TRUSS ELEVATION DRAWINGS. ALL BRIDGING AND THEIR END CONNECTIONS SHALL BE COMPLETELY INSTALLED BEFORE CONSTRUCTION LOADS ARE PLACED ON THE TRUSSES.

(b) DESCRIPTION OF WORK

- EXTENT OF LIGHT- GAUGE STEEL FRAMING IS SHOWN ON THE DRAWINGS.
- 2. TYPES OF LIGHT- GAUGE STEEL FRAMING MEMBERS TO BE "CSW", AND "TSB" SHAPED STRUCTURAL STEEL SHAPES, UNPUNCHED, BY DIETRICH INDUSTRIES, INC.

(c) QUALITY ASSURANCE

- 1. COMPONENT DESIGN: STRUCTURAL PROPERTIES OF LIGHT- GAUGE STUDS AND JOISTS WERE COMPUTED IN ACCORDANCE WITH AISI "SPECIFICATIONS FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS", LATEST EDITION AND MOST RECENT AMENDMENTS.
- 2. FIRE-RATED ASSEMBLIES: WHERE STEEL FRAMING MEMBERS ARE COMPONENTS OF ASSEMBLIES INDICATED FOR A FIRE-RESISTANCE RATING, THE CONTRACTOR SHALL VERIFY, FOR COMPLIANCE WITH GOVERNING REGULATIONS, WHICH MEMBERS PROVIDED HAVE BEEN APPROVED BY THE GOVERNING AUTHORITIES HAVING JURISDICTION.
- 3. SYSTEM SHALL BE INSTALLED BY PERSONNEL EXPERIENCED IN LIGHT-GAUGE STEEL FRAMING INSTALLATION. ERECTORS CAN PANELIZE WALLS WITH APPROVAL FROM THE ENGINEER.

(d) SUBMITTALS

- 1. PRODUCT DATA: SUBMIT MANUFACTURER PROJECT INFORMATION ON LIGHT- GAUGE STEEL FRAMING AND ACCESSORIES, INCLUDING OTHER DATA AS MAY BE REQUIRED TO CERTIFY COMPLIANCE WITH PERFORMANCE AND PRODUCT REQUIREMENTS SPECIFIED HEREIN.
- 2. SHOP DRAWINGS: NO SHOP DRAWINGS ARE REQUIRED TO BE SUBMITTED. THE DESIGN DOCUMENTS HEREIN SHALL BE SUFFICIENT FOR CONSTRUCTION. ANY QUESTIONS ON THE SCOPE OF WORK OR NOTES, OR ANYTHING ELSE CAN BE DIRECTED TO THE ENGINEER FOR CLARIFICATION AND/OR COORDINATION.

D. PRODUCTS

- 1. SYSTEM COMPONENTS: WITH EACH TYPE OF STEEL FRAMING REQUIRED, PROVIDE MANUFACTURER'S STANDARDS STEEL RUNNERS (TRACKS), BLOCKING, LINTELS, CLIP ANGLES, BRACING, REINFORCEMENTS, FASTENERS, AND ACCESSORIES AS RECOMMENDED BY THE MANUFACTURER FOR APPLICATIONS INDICATED, AS NEEDED TO PROVIDE A COMPLETE STEEL FRAMING SYSTEM, AND/OR AS INDICATED IN THESE DRAWINGS.
- FABRICATE METAL FRAMING COMPONENTS OF STRUCTURAL QUALITY STEEL SHEETS WITH A MINIMUM YIELD POINT OF 50,000 PSI FOR STUDS AND GUSSET PLATES, AND 50,000 PSI FOR RUNNERS.
- 3. FASTENERS SHALL BE BY "HILTI" AS SHOWN ON THE DRAWINGS. NO SUBSTITUTIONS ALLOWED.
- 4. PROVIDE GALVANIZED FINISH TO METAL FRAMING COMPONENTS COMPLYING WITH ASTM A-525 WITH A G-60 COATING.
- 5. "CSW" SHAPE STUDS: MANUFACTURER STANDARD STRUCTURAL STEEL STUDS OF SIZE, SHAPE, AND GAUGE INDICATED, WITH A 2-INCH FLANGE, AND 5/8" LIP RETURN.
- TSB" SHAPE STUDS: MANUFACTURER STANDARD STRUCTURAL STEEL TRACK OF SIZE, SHAPE, AND GAUGE INDICATED, WITH A 1 1/4-INCH FLANGE.

LIGHT-GAUGE NOTES (Continued on S5.6)

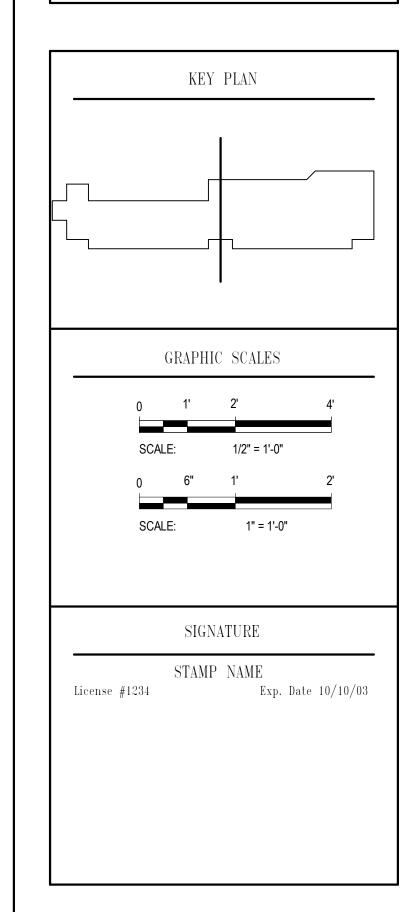
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ROOF TRUSS T-21 ELEVATION & DETAILS

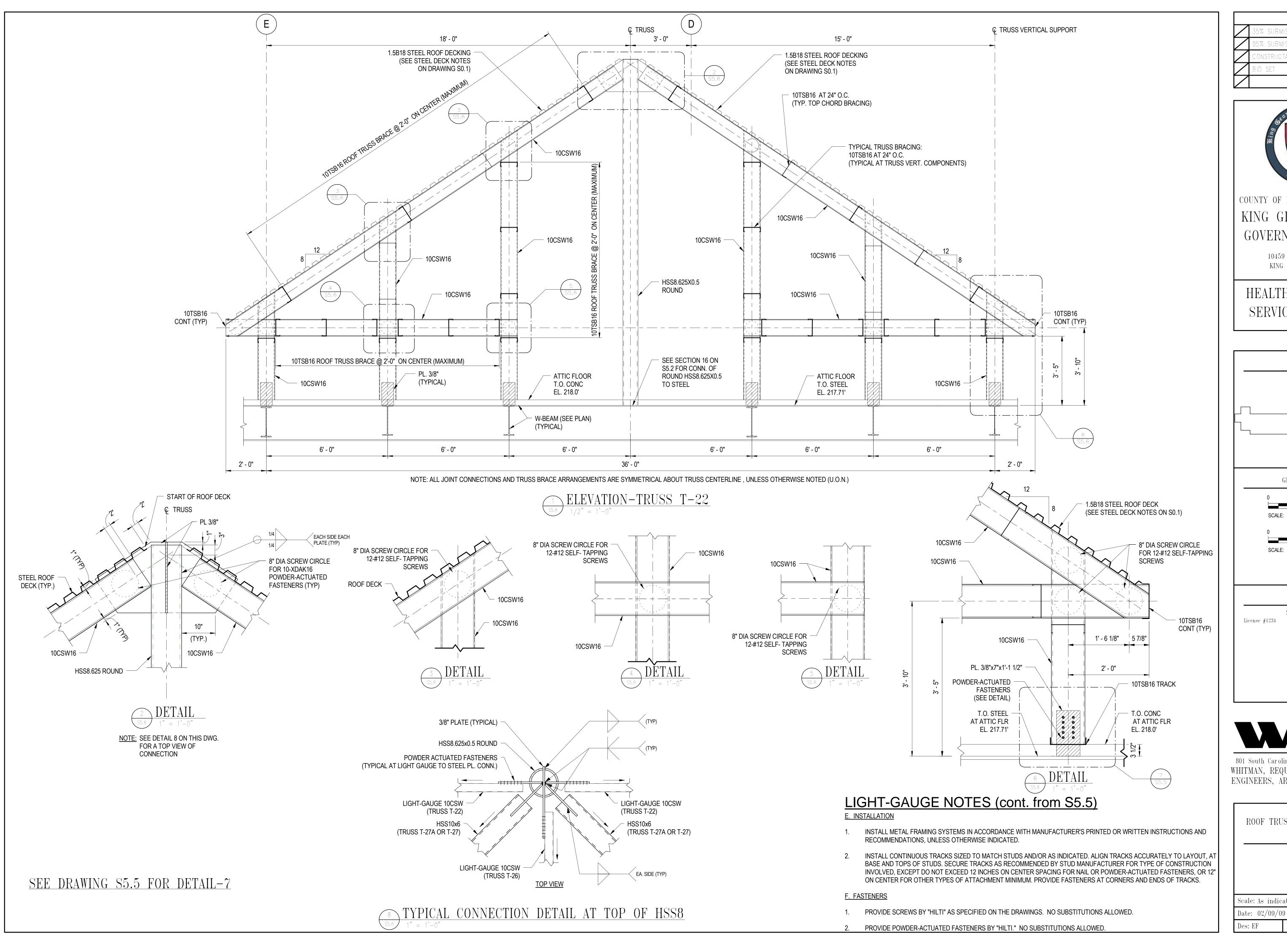
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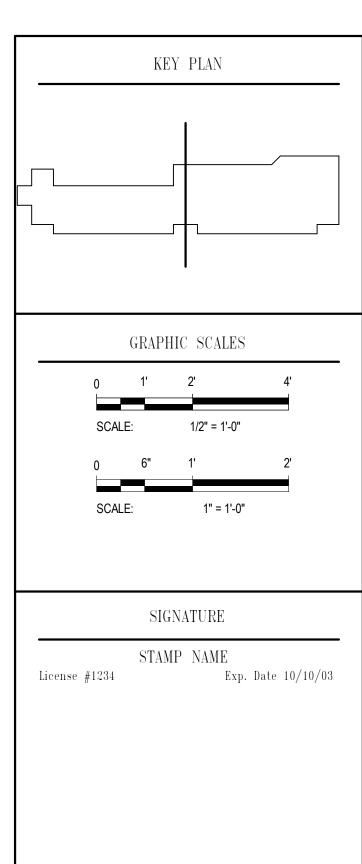


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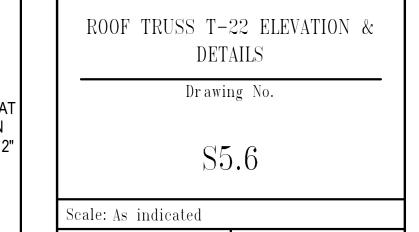
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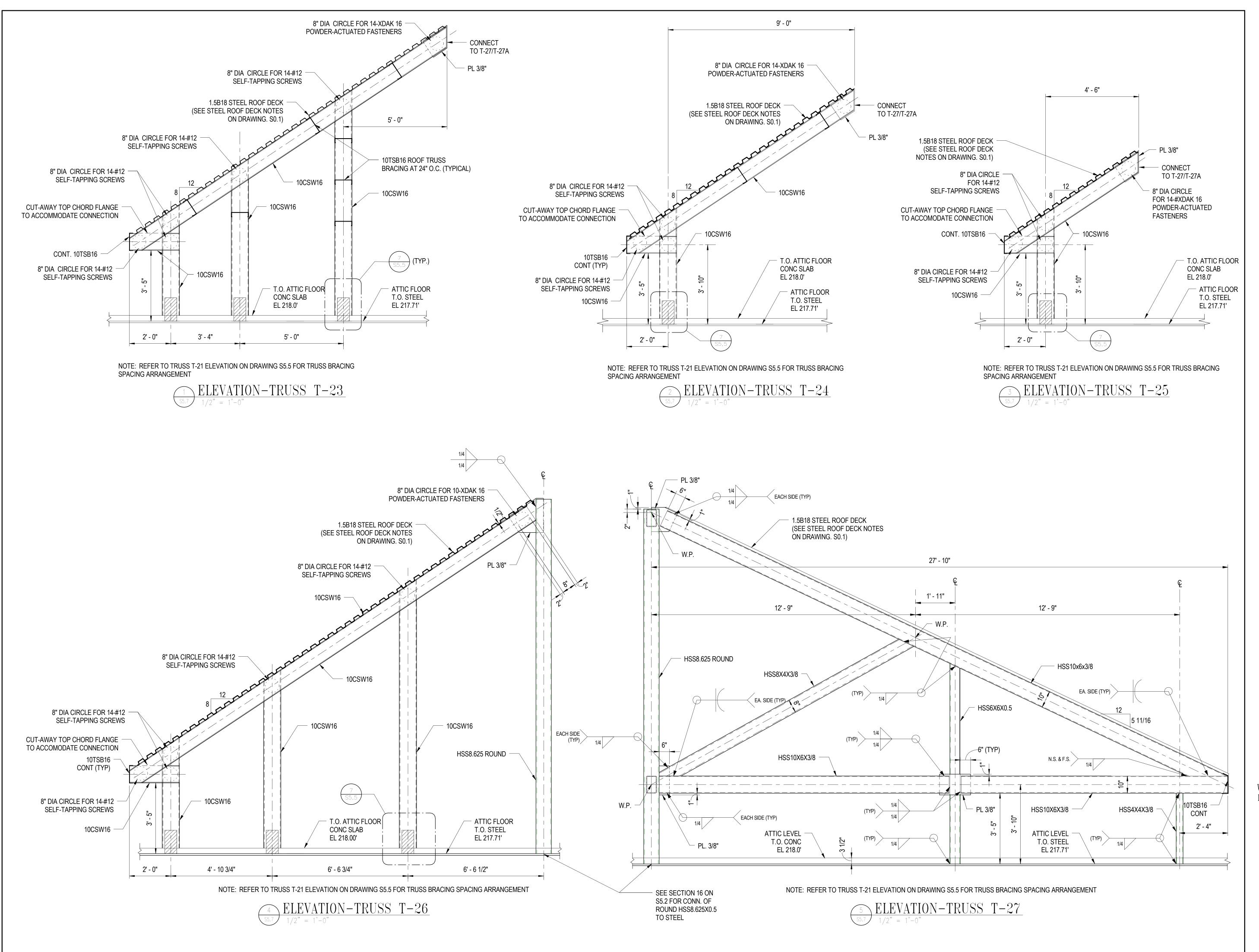


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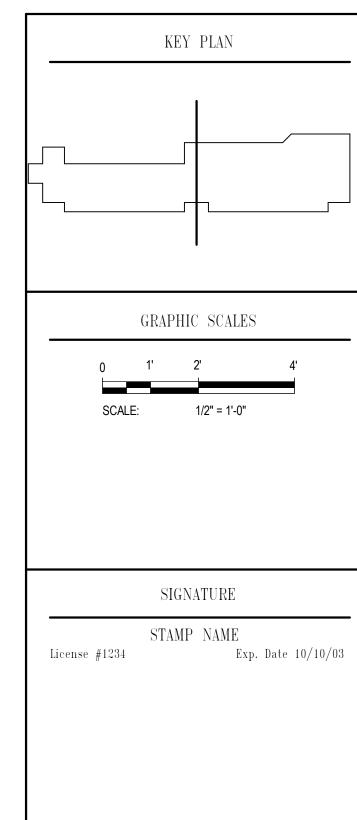


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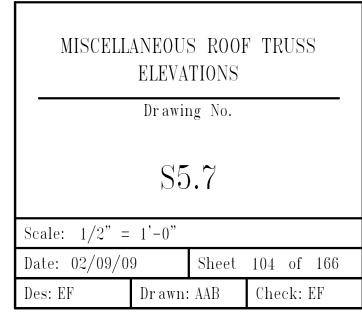


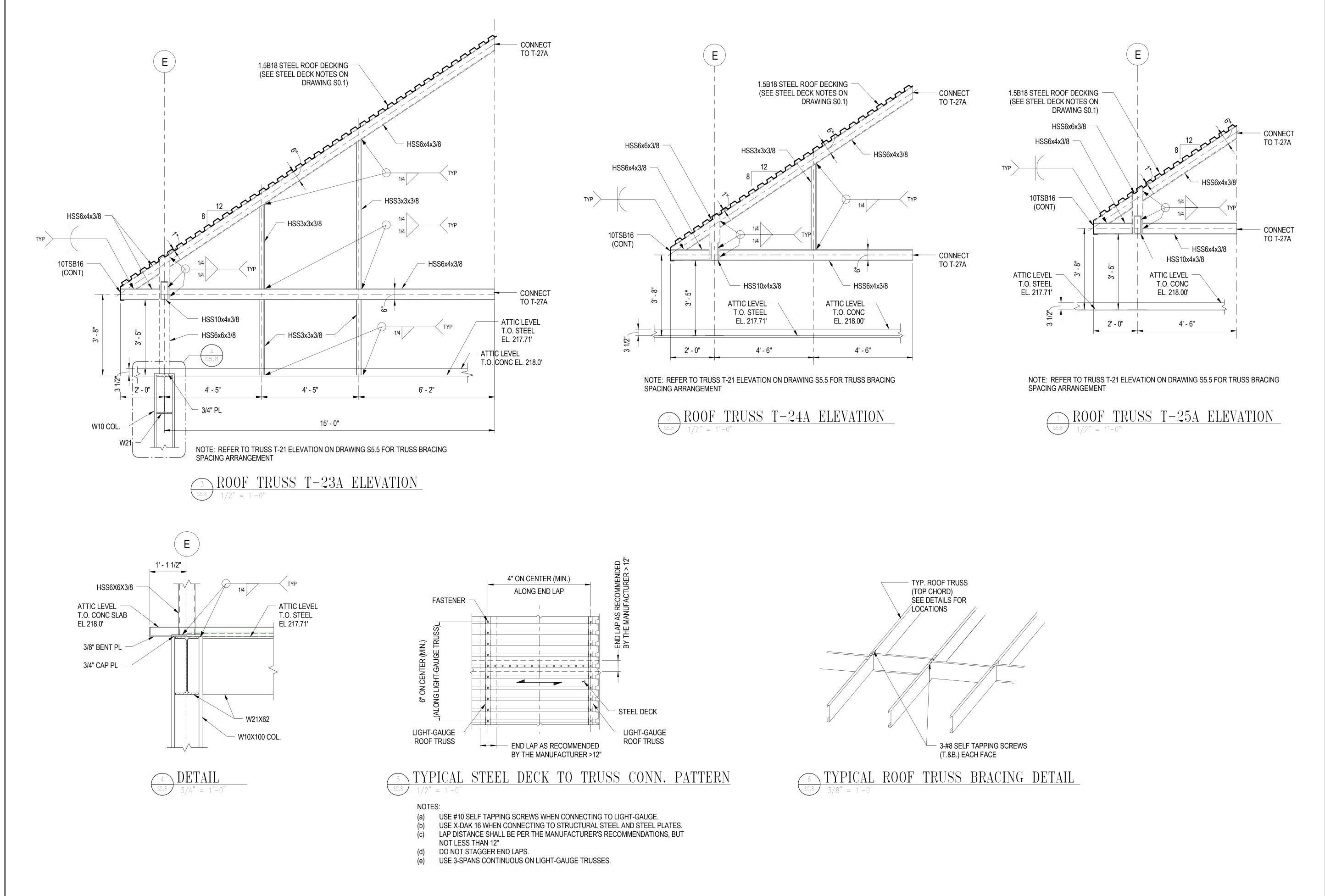
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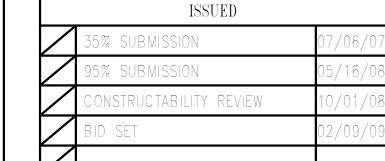
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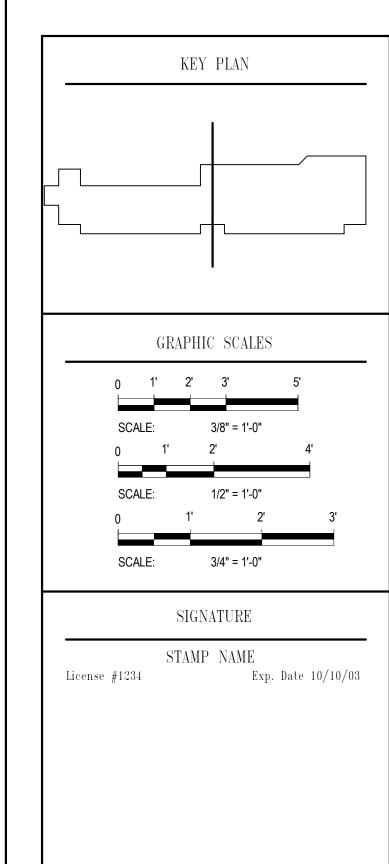






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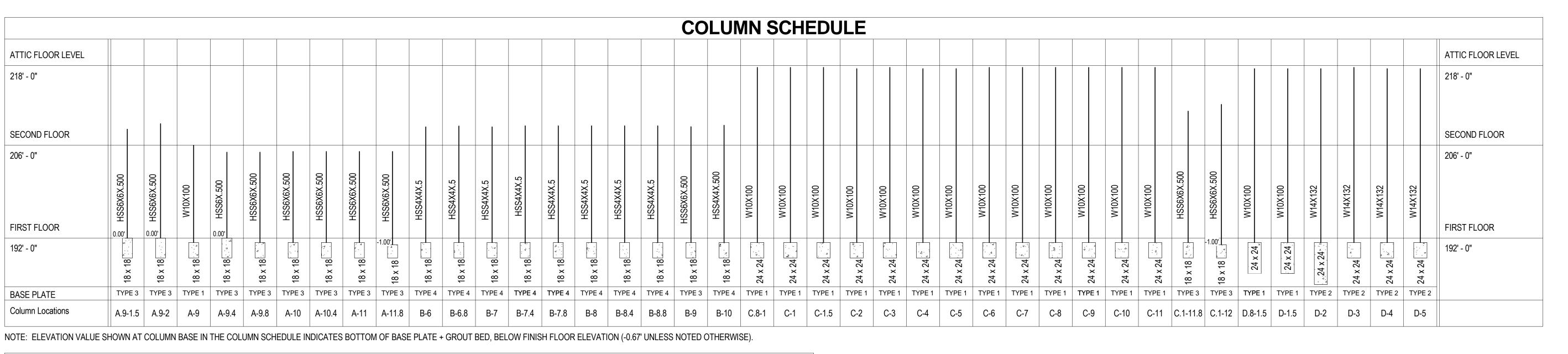
MISCELLANEOUS ROOF TRUSS
ELEVATIONS AND DETAILS

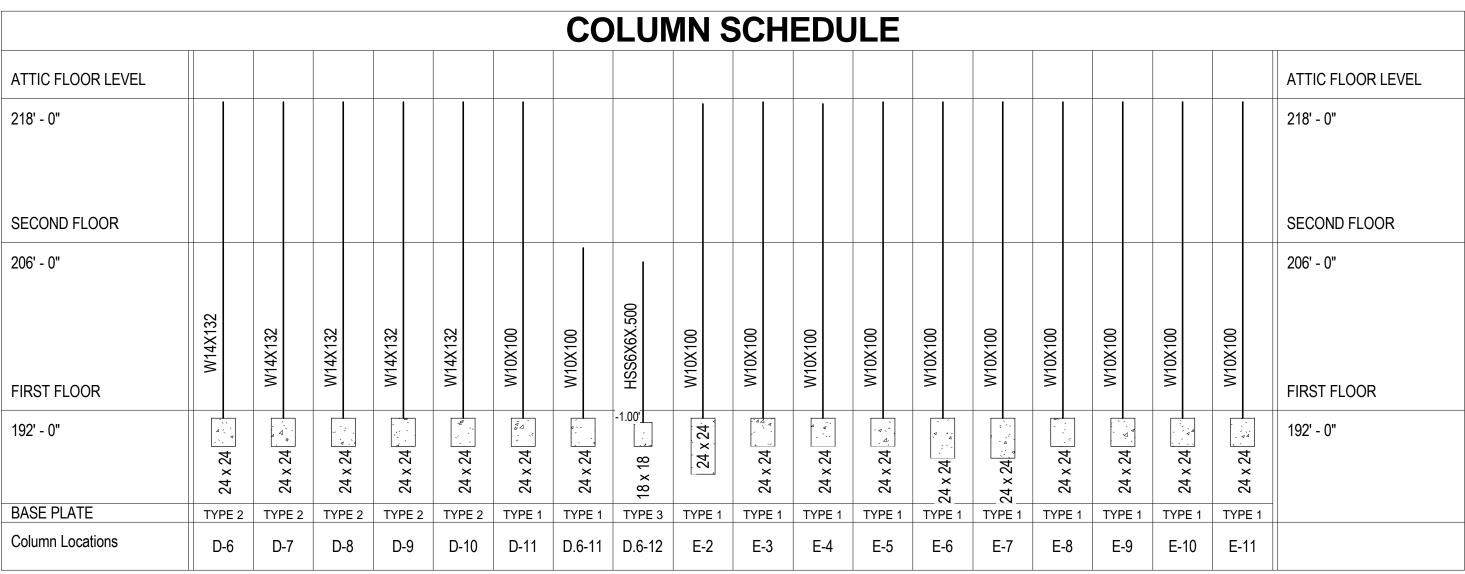
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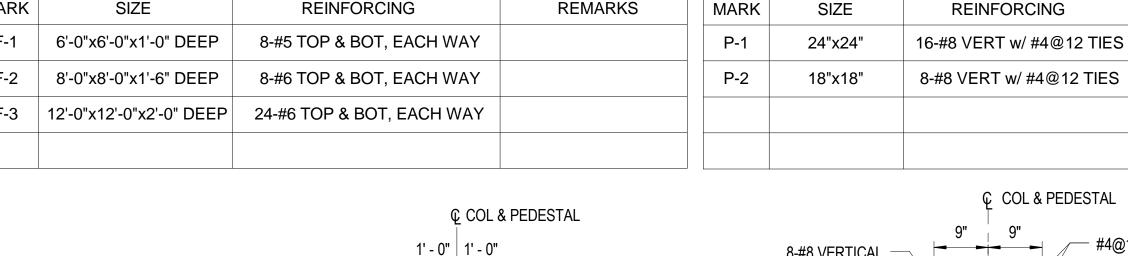
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NOTE: ELEVATION VALUE SHOWN AT COLUMN BASE IN THE COLUMN SCHEDULE INDICATES BOTTOM OF BASE PLATE + GROUT BED, BELOW FINISH FLOOR ELEVATION (-0.67' UNLESS NOTED OTHERWISE).

ATTIC FLOOR LEVEL	MARK	
218' - 0"	F-1	6'-0'
	F-2	8'-0'
	F-3	12'-0'
SECOND FLOOR		
206' - 0"		
FIRST FLOOR		
192' - 0"		

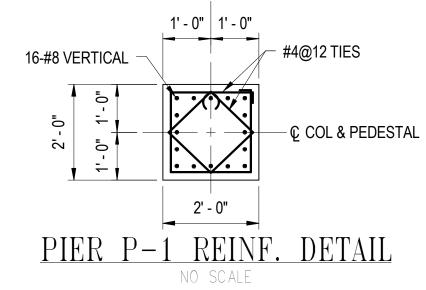


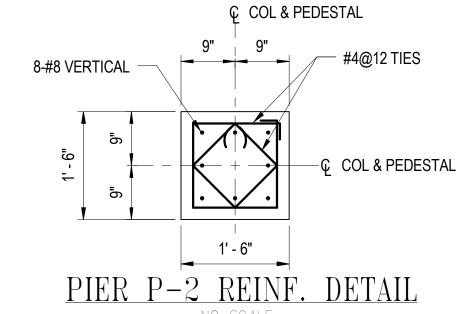
REMARKS

FOOTING SCHEDULE

REINFORCING

SIZE

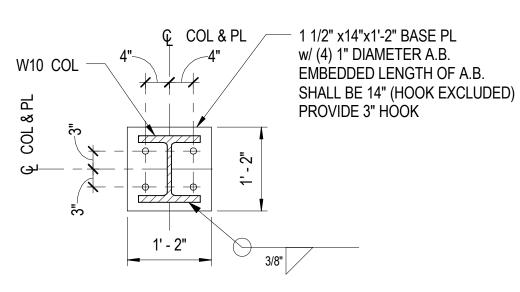


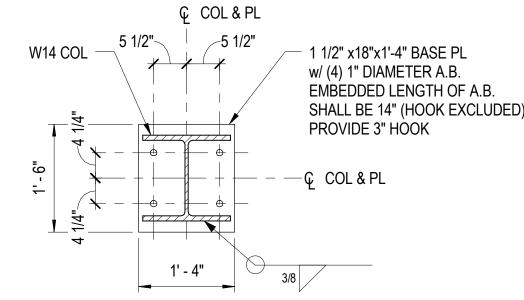


PIER SCHEDULE

REINFORCING

REMARKS





	Ç COL&PL ∕	1 1/2" x14"x1'-2" BASE PL
W10 COL — 4"-	4"	w/ (4) 1" DIAMETER A.B.
W10 00L	* * * /	EMBEDDED LENGTH OF A.B.
7		SHALL BE 14" (HOOK EXCLUDE
∞ ₋		PROVIDE 3" HOOK
COL & PL		
О + -		
<u> Э</u> — — — — — — — — — — — — — — — — — — —		
/ - - -		
<u>ب</u> –		
	41 011	
_	1' - 2"	8"
		• /

BASE PLATE - TYPE 1

NO SCALE

⊊ COL&PL



TOP OF 8" — CONCRETE SLAB	STEEL COLUMN (SEE SCHEDULE) BASE PLATE (SEE SCHEDULE) W/ (4) 1" DIAMETER ANCHOR BOLTS AND 3" HOOK WITH 14" EMBEDMENT	/— STEEL COLUMN (SEE SCHEDULE)
1" NON-SHRINK GROUT	#8 VERT (TYP)	TOP OF 8" CONCRETE SLAB TOP OF 8" W/ (4) 1" DIAMETER ANCHOR BOLTS AND 3" HOOK WITH 14" EMBEDMENT #8 VERT (TYP)
CONC PIER	#4 TIES (TYP)	1" NON-SHRINK GROUT (TYPICAL) CONC PIER #4 TIES (TYP)

FOR FOOTING SIZE -AND REINFORCING

SEE SCHEDULE

(SEE FOOTING SCHEDULE)

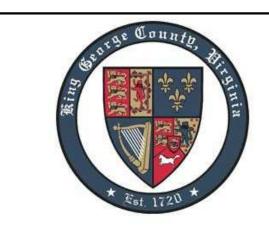
PIER TYPE -2 ELEVATION

Ç COL & PL	
3 1/2" 1"x12"x1'-0" BASE PLATE W/ (4) 1" DIAMETER ANCHOR BOLT w/3" HOOK AND 14" EMBEDMENT C COL & PL 1'-0"	

HSS6x6 COL 4 1/2" 4 1/2" PIPE 6" ROUND	 1"x14"x1'-2" BASE PLATE w/ (4) 1" DIAMETER ANCHOR BOLTS w/3" HOOK AND 14" EMBEDMENT
1-2-1-2-1-3	COL & PL
1' - 2"	3/8
→	
<u>BASE PLATE – T</u>	<u>YPE 3</u>

BASE PLATE - TYPE 4

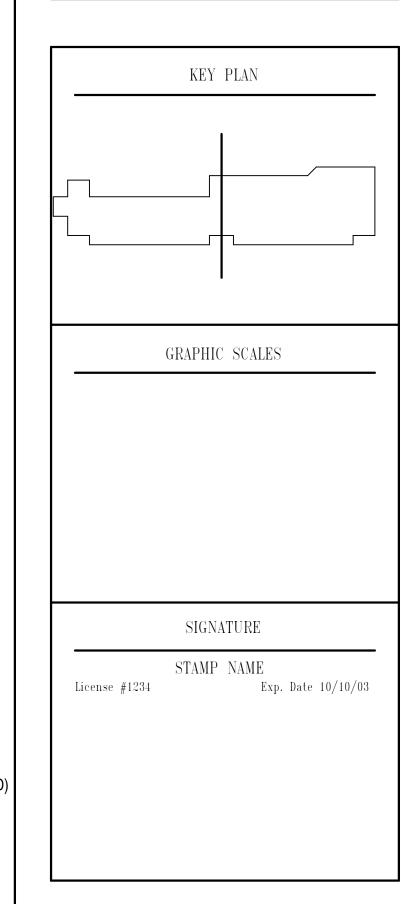
		ISSUED			
		35% SUBMISSION	07/06/07		
		95% SUBMISSION	05/16/08		
		CONSTRUCTABILITY REVIEW	10/01/08		
		BID SET	02/09/09		
		1			



COUNTY OF KING GEORGE, VIRGINIA KING GEORGE COUNTY GOVERNMENT CENTER

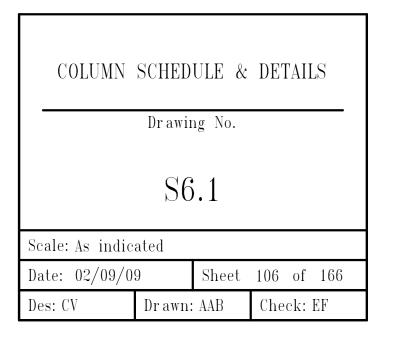
> 10459 COURTHOUSE ROAD KING GEORGE, VA 22485

HEALTH AND HUMAN SERVICES BUILDING





801 South Caroline Street, Baltimore, MD 21231 WHITMAN, REQUARDT & ASSOCIATES, LLP ENGINEERS, ARCHITECTS AND PLANNERS

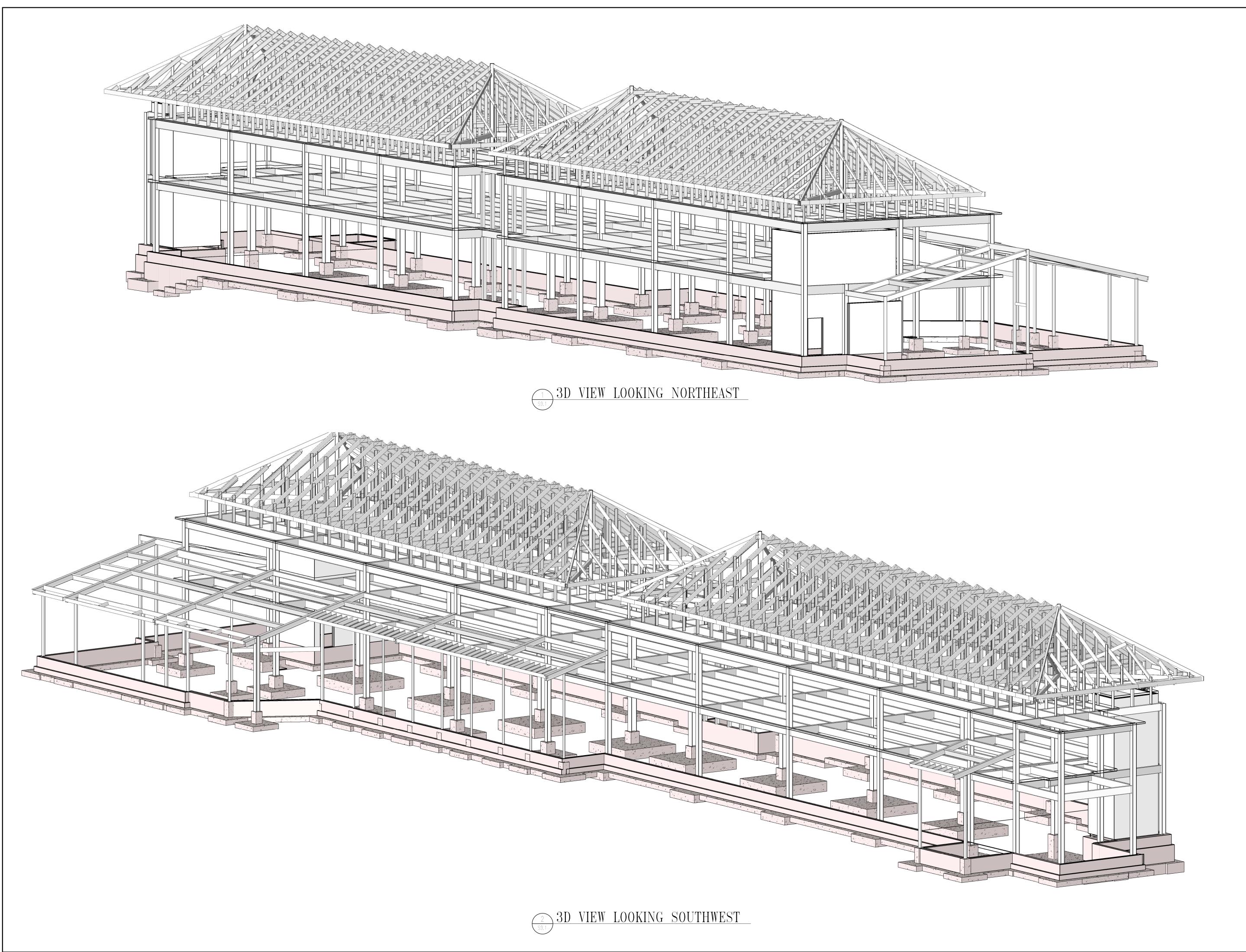


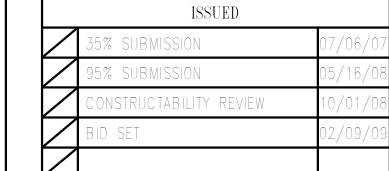
FOR FOOTING SIZE AND REINFORCING

SEE SCHEDULE

(SEE FOOTING SCHEDULE)

PIER TYPE -1 ELEVATION

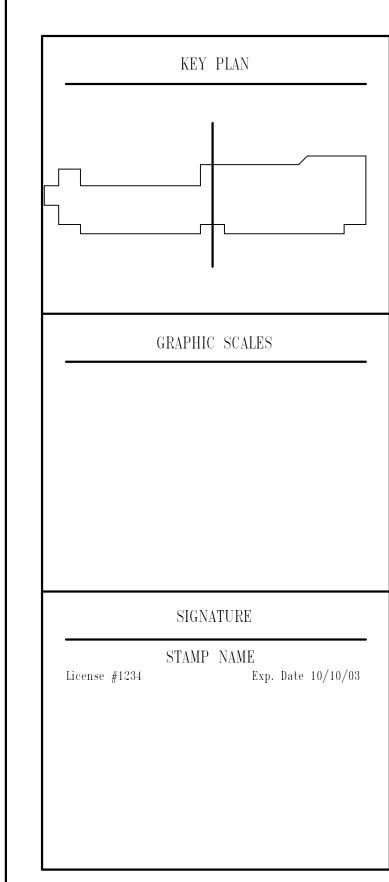






10459 COURTHOUSE ROAD KING GEORGE, VA 22485

HEALTH AND HUMAN SERVICES BUILDING





	3D VIEWS							
	Dr awing No.							
	S9.1							
S	Scale:							
	Date: 02/09/09		Sheet	107 of 166				
	es: CV	Dr awn:	AAB	Check: EF				